STATE OF LOUISIANA
DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT

RE-ADVERTISEMENT
CONSTRUCTION PROPOSAL

STATE PROJECT NO. 407-01-0046

GALLIANO BRIDGE
GEARBOX REPAIRS

LOCATION: LAFOURCHE PARISH
LA 308 & BAYOU LAFOURCHE
STATE PROJECT NO. 407-01-0046
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Electronic bids and electronic bid bonds for the following project will be downloaded by the Department of Transportation and Development (DOTD) on Wednesday, January 28, 2009. **Paper bids and paper bid bonds will not be accepted.** Electronic bids and electronic bid bonds must be submitted through [www.bidx.com](http://www.bidx.com) prior to the electronic bidding deadline. Beginning at 10:00 a.m., all bids will be downloaded and posted online at [http://www.dotd.la.gov/cgi-bin/construction.asp](http://www.dotd.la.gov/cgi-bin/construction.asp). No bids are accepted after 10:00 a.m.

**STATE PROJECT NO. 407-01-0046**
DESCRIPTION: Repairs to Galliano Bridge gearboxes and driveline
ROUTE: LA 308
PARISH: Lafourche Parish
ESTIMATED COST RANGE: $500,000 to $1,000,000
PROJECT ENGINEER: Keith Angelette, P.E., 5056 West Main St., Houma, LA 70360
   Phone: (985) 858-2403.
PROJECT MANAGER: John Harter, P.E., P.O. Box 94245, Baton Rouge, LA 70804
   Phone: (225) 379-1558.

Bids must be prepared and submitted in accordance with Section 102 of the 2006 Louisiana Standard Specifications for Roads and Bridges as amended by the project specifications, and must include all information required by the proposal.
NOTICE TO CONTRACTORS (CONTINUED)

Paper plans and/or proposals may be obtained in Room 101-A of the DOTD Headquarters Administration Building, 1201 Capitol Access Road in Baton Rouge, or by contacting the DOTD; Email: sharonknight@dotd.la.gov, Phone (225) 379-1111, FAX: (225) 379-1714, or by written requests sent to the Louisiana Department of Transportation and Development, Project Control Section, P. O. Box 94245, Baton Rouge, LA 70804-9245. Proposals will not be issued later than 24 hours prior to the time set for opening bids. All Addenda, Amendments, Letters of Clarification, and Withdrawal Notices will be posted online. Paper notices will not be distributed. Construction proposal information may be accessed via the Internet at www.dotd.la.gov. From the LA DOTD home page, select the following options: Doing Business with DOTD, then Construction Letting Information. Once the Construction Letting Information page appears, find the Notice to Contractors box. From the drop down menu, select the appropriate letting date and press the “Go To button to open the page, which provides a listing of all projects to be let and a Construction Proposal Documents link for each project. All project specific notices are found here. It will be the responsibility of the bidder to check for updates. If paper copies of the proposal are desired, the proposal cost is $25.00. Paper copies of the plans are included in the proposal (no additional charge). The purchase price for paper plans and proposals is non-refundable. Additionally, plans and specifications may be seen at the Project Engineer's office or in Room 101-A of the DOTD's Headquarters Administration Building in Baton Rouge. Upon request, the Project Engineer will show the work.

All questions concerning the plans shall be submitted via the Electronic Plans Distribution Center known as Falcon. Questions submitted within 96 hours of the bid deadline may not be answered prior to bidding. Falcon may be accessed via the Internet at www.dotd.la.gov. From the home page, select Doing Business with DOTD from the left-hand menu, then select Construction Letting Information on the pop-up menu. On the Construction Letting Information page, select the link, DOTD’s Plan Room. Login to Falcon (or request an ID if a first-time user). Once logged in, you will have access to view Project Information, submit a question concerning the project, and view the plans. All submitted questions will be forwarded by email to the Project Manager and the Project Engineer for a response.

The U. S. Department of Transportation (DOT) operates a toll free "Hotline" Monday through Friday, 8:00 a.m. to 5:00 p.m., eastern time. Anyone with knowledge of possible bid rigging, bidder collusion, or other fraudulent activities should call 1-800-424-9071. All information will be treated confidentially and caller anonymity will be respected.
GENERAL BIDDING REQUIREMENTS (08/06): The specifications, contract and bonds governing the construction of the work are the 2006 Edition of the Louisiana Standard Specifications for Roads and Bridges, together with any supplementary specifications and special provisions attached to this proposal.

Bids shall be prepared and submitted in accordance with Section 102 of the Standard Specifications.

The plans herein referred to are the plans approved and marked with the project number, route and Parish, together with all standard or special designs that may be included in such plans. The bidder declares that the only parties interested in this proposal as principals are those named herein; that this proposal is made without collusion or combination of any kind with any other person, firm, association, or corporation, or any member or officer thereof; that careful examination has been made of the site of the proposed work, the plans, Standard Specifications, supplementary specifications and special provisions above mentioned, and the form of contract and payment, performance, and retainage bond; that the bidder agrees, if this proposal is accepted, to provide all necessary machinery, tools, apparatus and other means of construction and will do all work and furnish all material specified in the contract, in the manner and time therein prescribed and in accordance with the requirements therein set forth; and agrees to accept as full compensation therefore, the amount of the summation of the products of the quantities of work and material incorporated in the completed project, as determined by the engineer, multiplied by the respective unit prices herein bid.

It is understood by the bidder that the quantities given in this proposal are a fair approximation of the amount of work to be done and that the sum of the products of the approximate quantities multiplied by the respective unit prices bid shall constitute gross sum bid, which sum shall be used in comparison of bids and awarding of the contract.

The bidder further agrees to perform all extra and force account work that may be required on the basis provided in the specifications.

The bidder further agrees that within 15 calendar days after the contract has been transmitted to him, he will execute the contract and furnish the Department satisfactory surety bonds.

If this proposal is accepted and the bidder fails to execute the contract and furnish bonds as above provided, the proposal guaranty shall become the property of the Department; otherwise, said proposal guaranty will be returned to the bidder; all in accordance with Subsection 103.04.

MANDATORY ELECTRONIC BIDS AND ELECTRONIC BID BONDS SUBMISSION (10/08): This project requires mandatory electronic bidding. All Specifications, whether Standard, Supplemental or Special Provisions, are hereby amended to delete any references regarding paper bids and the ability to submit paper bid forms.

The contractor shall register online to be placed on the Louisiana Department of Transportation and Development (LA DOTD) prospective bidders list or for information only list.

Modifications to proposal documents will be posted on the Department’s website at the following URL address: www.dotd.la.gov/cgi-bin/construction.asp.

LA DOTD shall not be responsible if the bidder cannot complete and submit a bid due to failure or incomplete delivery of the files submitted via the internet.
MAINTENANCE OF TRAFFIC (08/06): Subsection 104.03 of the 2006 Standard Specifications is amended to include the following requirements.

In order to maintain traffic, the contractor shall construct temporary detours as required by the contract.

CONTRACT TIME (10/01): The contractor will be issued a "Conditional Notice to Proceed" as defined in Subsection 101.03. The "Conditional Notice to Proceed" will expire **one hundred twenty (120) calendar days** after its issuance, whereupon a "Notice to Proceed" will become effective, unless the contractor begins regular construction at an earlier date at which time the Notice to Proceed becomes effective.

The entire contract shall be completed in all details and ready for final acceptance in accordance with Subsection 105.17(b) within **THIRTY-FIVE (35) WORKING DAYS** after the effective date of the "Notice to Proceed".
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LOUISIANA
DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT
SUPPLEMENTAL SPECIFICATIONS

The 2006 Louisiana Standard Specifications for Roads and Bridges and supplemental specifications thereto are amended as follows.

PART I – GENERAL PROVISIONS

SECTION 101 – GENERAL INFORMATION, DEFINITIONS, AND TERMS:
Subsection 101.03 – Definitions (07/07), Pages 3 – 13).
Delete the definition for “Proposal/Bid Guaranty” and substitute the following.
Proposal / Bid Guaranty.  The required security furnished with a bid.  The only form of security acceptable is a Bid Bond.

SECTION 102 – BIDDING REQUIREMENTS:
Subsection 102.09 – Proposal / Bid Guaranty (07/07), Page 19.
Delete the contents of this subsection and substitute the following.
PROPOSAL/BID GUARANTY.  Each bid shall be accompanied by a proposal/bid guaranty in an amount not less than five percent of the total bid amount when the bidder’s total bid amount as calculated by the Department in accordance with Subsection 103.01 is greater than $50,000.  No proposal/bid guaranty is required for projects when the bidder’s total bid amount as calculated by the Department is $50,000 or less.  The official total bid amount for projects that include alternates is the total of the bidder's base bid and all alternates bid on and accepted by the Department.  The proposal/bid guaranty submitted by the bidder shall be a bid bond made payable to the contracting agency as specified on the bid bond form provided in the construction proposal.  No other form of security will be accepted.

The bid bond shall be on the "Bid Bond" form provided in the construction proposal, on a form that is materially the same in all respects to the "Bid Bond" form provided, or on an electronic form that has received Department approval prior to submission.  The bid bond shall be filled in completely, shall be signed by an authorized officer, owner or partner of the bidding entity, or each entity representing a joint venture; shall be signed by the surety's agent or attorney-in-fact; and shall be accompanied by a notarized document granting general power of attorney to the surety's signer.  The bid bond shall not contain any provisions that limit the face amount of the bond.

The bid bond will be written by a surety or insurance company that is in good standing and currently licensed to write surety bonds in the State of Louisiana by the Louisiana Department of Insurance and also conform to the requirements of LSA-R.S. 48:253.

All signatures required on the bid bond may be original, mechanical reproductions, facsimiles or electronic.  Electronic bonds issued in conjunction with electronic bids must have written Departmental approval prior to use.  The Department will make a listing of approved electronic sureties providers on the Bidx.com site.
SECTION 107 – LEGAL RELATIONS AND RESPONSIBILITY TO PUBLIC:
Subsection 107.05 – Federal Aid Participation (04/08), Pages 57 and 58.
   Delete the second paragraph.

SECTION 108 – PROSECUTION AND PROGRESS:
Subsection 108.04 – Prosecution of Work (03/05) Pages 74 and 75.
   Add the following sentence to the third paragraph of Heading (b).
   Should the surety or the Department take over prosecution of the work, the contractor
   shall remain disqualified for a period of one year from the completion of the project, unless
   debarment proceedings are instituted.
   When the Department of Transportation and Development is not the contracting agency
   on the project, the second paragraph under Heading (c) is deleted.

PART II – EARTHWORK

SECTION 202 – REMOVING OR RELOCATING STRUCTURES AND OBSTRUCTIONS:
Subsection 202.06 – Plugging or Relocating Existing Water Wells (03/04), Page 105.
   Delete the first sentence and substitute the following.
   All abandoned wells shall be plugged and sealed at the locations shown on the plans, or
   as directed by the engineer, in accordance with the “Water Well Rules, Regulations, and
   Standards, State of Louisiana.” This document is available at the Department of Transportation
   and Development, Water Resources Section, P. O. Box 94245, Baton Rouge, Louisiana 70804-
   9245. The Water Resource Section’s telephone number is (225) 274-4172.

PART III – BASE COURSES

SECTION 302 – CLASS II BASE COURSE:
Subsection 302.05 – Mixing (08/06), Pages 152 and 153.
   Delete the first sentence of Subheading (b)(1), In-Place Mixing, and substitute the
   following.
   In-place mixing shall conform to Heading (a)(1) except that the percentage of Type I
   portland cement required will be 6 percent by volume.

SECTION 305 – SUBGRADE LAYER:
Subsection 305.06 – Payment (01/08), Page 184.
   Delete the contents of this subsection and substitute the following.
   305.06 Payment. Payment for subgrade layer will be made at the contract unit price
   which includes lime, lime treatment, cement, cement treatment, water, stone, recycled portland
   cement concrete, crushed slag, blended calcium sulfate, asphaltic concrete, and asphalt curing
   membrane or prime coat, subject to the payment adjustment provisions of Section 1002 for
   specification deviations of asphalt materials and Subsection 303.11(a) for density deficiencies of
   cement treated materials. Adjustments in pay for increase or decrease in the percent cement
   ordered by the engineer will be in accordance with Subsection 303.13. Adjustments in pay for
increase or decrease in the percent lime ordered by the engineer will be based on the price of lime shown on paid invoices (total of all charges). The Materials and Testing Section will provide the payment adjustment percentage for properties of asphalt materials.

Payment for geotextile fabric will be included in the contract unit price for subgrade layer.

Payment will be made under:

<table>
<thead>
<tr>
<th>Item No.</th>
<th>Pay Item</th>
<th>Pay Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>305-01</td>
<td>Subgrade Layer</td>
<td>_____ in (mm) Thick</td>
</tr>
</tbody>
</table>

**SECTION 307 – PERMEABLE BASES:**
Subsection 307.02 – Materials (09/07), Pages 187 and 188.

Delete the contents of Subheading (b), Asphalt, and substitute the following.

(b) Asphalt: The asphalt for asphalt treated permeable base shall be an approved polymer modified asphalt cement, PG 76-22m, or PG 82-22rm complying with Section 1002. The percentage of asphalt cement shall be 2.0 percent to 4.0 percent by weight (mass) of the total mixture. Asphalt cement content and mixing process shall be such that all aggregates are visibly coated. The mixture shall retain 90 percent coating when tested in accordance with DOTD TR 317.

A job mix formula shall be submitted and approved in accordance with Section 502.

**SECTION 308 – IN-PLACE CEMENT TREATED BASE COURSE:**
All Subsections within Section 308 – (07/07), Pages 191 – 198.

Whenever the reference to “DOTD TR-432, Method D” is used, it shall mean “DOTD TR-432”.

**PART V – ASPHALTIC PAVEMENTS**

**SECTION 502 – SUPERPAVE ASPHALTIC CONCRETE MIXTURES:**
Subsection 502.02 – Materials (08/06) (11/07), Pages 210 – 213.

Delete Table 502-2, Superpave Asphalt Cement Usage under Subheading (a) and substitute the following.
### Table 502-2

**Superpave Asphalt Cement Usage**

<table>
<thead>
<tr>
<th>Current Traffic Load Level</th>
<th>Mixture Type</th>
<th>Grade of Asphalt Cement</th>
</tr>
</thead>
<tbody>
<tr>
<td>Level 1</td>
<td>Wearing Course</td>
<td>PG 70-22m</td>
</tr>
<tr>
<td></td>
<td>Binder Course</td>
<td>PG 70-22m</td>
</tr>
<tr>
<td></td>
<td>Base Course</td>
<td>PG 64-22</td>
</tr>
<tr>
<td>Level 2</td>
<td>Wearing Course</td>
<td>PG 76-22m</td>
</tr>
<tr>
<td></td>
<td>Binder Course</td>
<td>PG 76-22m</td>
</tr>
<tr>
<td>Level A</td>
<td>Incidental Paving</td>
<td>PG 70-22m</td>
</tr>
</tbody>
</table>

Note: A PG 82-22 rm, Waste Tire Rubber Modified Asphalt, may be substituted for any other grade of asphalt cement.

Delete Table 502-3, Aggregate Friction Rating under Subheading (c)(1) and substitute the following.

### Table 502-3

**Aggregate Friction Rating**

<table>
<thead>
<tr>
<th>Friction Rating</th>
<th>Allowable Usage</th>
</tr>
</thead>
<tbody>
<tr>
<td>I</td>
<td>All mixtures</td>
</tr>
<tr>
<td>II</td>
<td>All mixtures</td>
</tr>
<tr>
<td>III</td>
<td>All mixtures, except travel lane wearing courses with plan ADT greater than 7000&lt;sup&gt;1&lt;/sup&gt;</td>
</tr>
<tr>
<td>IV</td>
<td>All mixtures, except travel lane wearing courses&lt;sup&gt;2&lt;/sup&gt;</td>
</tr>
</tbody>
</table>

<sup>1</sup> When plan current average daily traffic (ADT) is greater than 7000, blending of Friction Rating III aggregates and Friction Rating I and/or II aggregates will be allowed for travel lane wearing courses at the following percentages. At least 30 percent by weight (mass) of the total aggregates shall have a Friction Rating of I, or at least 50 percent by weight (mass) of the total aggregate shall have a Friction Rating of II. The frictional aggregates used to obtain the required percentages shall not have more than 10 percent passing the No. 8 (2.36 mm) sieve.

<sup>2</sup> When the average daily traffic (ADT) is less than 2500, blending of Friction Rating IV aggregates with Friction Rating I and/or II aggregates will be allowed for travel lane wearing courses at the following percentages. At least 50 percent by weight (mass) of the total aggregate in the mixture shall have a Friction Rating of I or II. The frictional aggregates used to obtain the required percentages shall not have more than 10 percent passing the No. 8 (2.36 mm) sieve.


Delete the first sentence of the first paragraph and substitute the following.
A lot is a segment of continuous production of asphaltic concrete mixture from the same job mix formula produced for the Department at a specific plant, delivered to a specific DOTD project.

SECTION 508 – STONE MATRIX ASPHALT:
Subsection 508.01 – Description (09/07), Page 274.
Delete this subsection and substitute the following.

508.01 DESCRIPTION. This work consists of furnishing and constructing Stone Matrix Asphalt (SMA) which is a plant mixed asphalt concrete wearing course for high traffic applications. This mixture is a rut resistant hot mix design with stone on stone contact. The mixture shall be composed of a PG 76-22m, or PG 82-22rm asphalt cement and a gap graded coarse aggregate structure. Mineral filler and/or fibers shall be used to control drain down. This work shall be in accordance with these specifications, plan details, and as directed. All requirements of Section 502 apply to Stone Matrix Asphalt, except as modified herein. All plant and paving equipment and processes must meet the requirements of Section 503.

Mixture used for shoulder may be Stone Matrix Asphalt or any mixture type shown in Table 502-5.

Subsection 508.02 – Materials (09/07), Page 274.
Delete the contents of subheading (a), Asphalt Cement and substitute the following.

(a) Asphalt Cement: Asphalt cement shall be PG 76-22m, or PG 82-22rm as listed on QPL 41 and complying with Section 1002.

PART VI – RIGID PAVEMENT

SECTION 602 – PORTLAND CEMENT CONCRETE PAVEMENT REHABILITATION:
Subsection 602.17 – Payment (09/07), Pages 341 – 344.
Delete the last paragraph of Subheadings (d), Full Depth Corner Patching of Jointed Concrete Pavement, (e) Full Depth Patching of Jointed Concrete Pavement, and (g) Patching Continuously Reinforced Concrete Pavement, and substitute the following.

Payment for deteriorated base course removed as directed by the engineer and replaced with concrete will be made as follows: The value per inch (mm) thickness will be determined by dividing the contract unit price per square yard (sq m) by the plan thickness. Thickness of patches will be measured from the surface that exists at the time of patching. Payment for the additional thickness will be made at 50 percent of the value per inch (mm) thus determined.

PART VII – INCIDENTAL CONSTRUCTION

SECTION 701 – CULVERTS AND STORM DRAINS:
All Subsections within Section 701 (08/07), Pages 347 – 358.
Delete Section 701, Culverts and Storm Drains and substitute the following.
SECTION 701
CULVERTS AND STORM DRAINS

701.01 DESCRIPTION. This work consists of furnishing, installing, and cleaning pipe, pipe arch, storm drains and sewers, also referred to as culverts or conduit, in accordance with these specifications and in conformity with lines and grades shown on the plans or established.

701.02 MATERIALS. Materials shall comply with the following sections and subsections:

<table>
<thead>
<tr>
<th>Material</th>
<th>Section/Subsection</th>
</tr>
</thead>
<tbody>
<tr>
<td>Usable Soil</td>
<td>203.06(a)</td>
</tr>
<tr>
<td>Selected Soil</td>
<td>203.06(b)</td>
</tr>
<tr>
<td>Plastic Soil Blanket</td>
<td>203.10</td>
</tr>
<tr>
<td>Mortar</td>
<td>702.02</td>
</tr>
<tr>
<td>Flowable Fill</td>
<td>710</td>
</tr>
<tr>
<td>Portland Cement Concrete</td>
<td>901</td>
</tr>
<tr>
<td>Reclaimed Asphalitic Pavement (RAP)</td>
<td>1003.01 &amp; 1003.04(d)</td>
</tr>
<tr>
<td>Stone</td>
<td>1003.03(b)</td>
</tr>
<tr>
<td>Recycled Portland Cement Concrete</td>
<td>1003.03(c)</td>
</tr>
<tr>
<td>Granular Material</td>
<td>1003.07</td>
</tr>
<tr>
<td>Bedding Material</td>
<td>1003.08</td>
</tr>
<tr>
<td>Concrete Sewer Pipe</td>
<td>1006.02</td>
</tr>
<tr>
<td>Reinforced Concrete Pipe</td>
<td>1006.03</td>
</tr>
<tr>
<td>Reinforced Concrete Pipe Arch</td>
<td>1006.04</td>
</tr>
<tr>
<td>Gasket Materials</td>
<td>1006.06</td>
</tr>
<tr>
<td>Plastic Pipe</td>
<td>1006.07</td>
</tr>
<tr>
<td>Split Plastic Coupling Bands</td>
<td>1006.07(d)(4)</td>
</tr>
<tr>
<td>Plastic Yard Drain Pipe</td>
<td>1006.09</td>
</tr>
<tr>
<td>Bituminous Coated Corrugated Steel Pipe and Pipe Arch</td>
<td>1007.02</td>
</tr>
<tr>
<td>Structural Plate for Pipe, Pipe Arch and Arch</td>
<td>1007.04</td>
</tr>
<tr>
<td>Corrugated Aluminum Pipe and Pipe Arch</td>
<td>1007.05</td>
</tr>
<tr>
<td>Coupling Bands</td>
<td>1007.09</td>
</tr>
<tr>
<td>Reinforcing Steel</td>
<td>1009</td>
</tr>
<tr>
<td>Geotextile Fabric</td>
<td>1019</td>
</tr>
</tbody>
</table>

(a) Side Drain Pipe or Side Drain Pipe Arch: When the item for Side Drain Pipe or Side Drain Pipe Arch is included in the contract, the contractor has the option of furnishing reinforced concrete pipe or reinforced concrete pipe arch, corrugated metal pipe or corrugated metal pipe arch, or plastic pipe, as allowed by EDSM II.2.1.1 or unless otherwise specified.

(b) Cross Drain Pipe or Cross Drain Pipe Arch: When the item for Cross Drain Pipe or Cross Drain Pipe Arch is included in the contract, the contractor has the option of furnishing reinforced concrete pipe or reinforced concrete pipe arch, corrugated metal pipe or corrugated metal pipe arch, or plastic pipe, as allowed by EDSM II.2.1.1 or unless otherwise specified.
(c) Storm Drain Pipe or Storm Drain Pipe Arch: When the item for Storm Drain Pipe or Storm Drain Pipe Arch is included in the contract, the contractor has the option of furnishing reinforced concrete pipe or reinforced concrete pipe arch, or plastic pipe, as allowed by EDSM II.2.1.1 or unless otherwise specified.

(d) Yard Drain Pipe: When the item for Yard Drain Pipe is included in the contract, the contractor has the option of furnishing concrete sewer pipe, plastic yard drain pipe or plastic pipe in accordance with Section 1006 unless otherwise specified.

(e) Material Type Abbreviations:
   (1) Reinforced Concrete Pipe:
       RCP    Reinforced Concrete Pipe
       RCPA   Reinforced Concrete Pipe Arch
   (2) Corrugated Metal Pipe:
       CAP    Corrugated Aluminum Pipe
       CAPA   Corrugated Aluminum Pipe Arch
       CMP    Corrugated Metal Pipe
       CMPA   Corrugated Metal Pipe Arch
       CSP    Corrugated Steel Pipe
       CSPA   Corrugated Steel Pipe Arch
       BCCSP  Bituminous Coated Corrugated Steel Pipe
       BCCSPA Bituminous Coated Corrugated Steel Pipe Arch
   (3) Plastic Pipe:
       PP     Plastic Pipe
       PVCP   Polyvinyl Chloride Pipe
       RPVCP  Ribbed Polyvinyl Chloride Pipe
       CPEPDW Corrugated Polyethylene Pipe Double Wall

(f) Joint Type Abbreviations:
   T1     Type 1 Joint
   T2     Type 2 Joint
   T3     Type 3 Joint

(g) Quality Assurance for Pipe: Manufacturing plants will be periodically inspected for compliance with specified manufacturing methods, and material samples will be randomly obtained for laboratory testing for verification of manufacturing lots. Materials approved at the manufacturing plant will be subject to visual acceptance inspections at the jobsite or point of delivery.

701.03 EXCAVATION. For all pipe, when the sides of the trench are stable as evidenced by the sides of the trench being able to maintain a vertical cut face, the minimum trench width at the bottom of the excavation will be 18 inches (460mm) on either side of the outside diameter of the pipe. If the sides of the trench are unstable, the width of the trench at the bottom of the excavation, for plastic or metal pipe, shall be a minimum width of at least 18 inches (460mm) or one pipe diameter on each side of the outside diameter of the pipe, whichever is greater. Surplus material or excavated material that does not conform to the requirements of Subsection 203.06(a) shall be satisfactorily disposed of in accordance with Subsection 202.02. Moisture controls
including backfill materials selection and dewatering using sumps, wells, well points or other approved processes may be necessary to control excess moisture during excavation, installation of bedding, over-excavated trench backfilling, pipe placement and pipe backfill.

(a) Over-excavation: When unsuitable soils as defined in Subsection 203.04 or a stable, non-yielding foundation cannot be obtained at the established pipe grade, or at the grade established for placement of the bedding, unstable or unsuitable soils below this grade shall be removed and replaced with granular material meeting the requirements of Subsection 1003.07, bedding materials meeting the requirements of Subsection 1003.08 or Type A backfill. All granular, backfill materials placed below the established pipe or bedding grade shall be placed in lifts not exceeding 8 inches (200 mm) thick and sufficiently compacted by hand or a dynamic mechanical hand compaction device over the surface of each lift to form a stable, non-yielding foundation at the surface of the established bedding or pipe grade.

When rock is encountered, it shall be removed below grade and replaced with material complying with Subsection 1003.07, bedding materials meeting the requirements of Subsection 1003.08 or Type A backfill. The compacted earth cushion shall have a thickness under the pipe of at least 1/2 inch per foot (40 mm/m) of fill height over the top of the pipe with a minimum thickness of 8 inches (200 mm). All granular, backfill materials placed below the established pipe or bedding grade shall be placed in lifts not exceeding 8 inches (200 mm) thick and sufficiently compacted by hand or a dynamic mechanical hand operated compaction device over the surface of each lift to form a stable, non-yielding foundation at the surface of the established bedding or pipe grade.

Materials used to backfill in an over-excavated portion of a trench do not require encasement in a Geotextile Fabric.

Density of approved materials placed in over-excavated trenches will not be measured or determined.

701.04 FORMING PIPE BED. Bedding material, when specified, shall be constructed in accordance with Section 726. Materials allowed for bedding shall be as specified in Subsection 1003.08 or may be Type A backfill materials. When bedding materials are specified, additional excavation shall be performed below established pipe grade and the bedding material placed in lifts not exceeding 8 inches (200 mm) thick and lightly compacted by hand or a dynamic hand compaction device over the surface of each lift.

When the bottom of the pipe is not laid in a trench but is constructed above natural soils, a uniform bed shall be constructed as specified for the bottom of a trench.

Density of approved bedding materials will not be measured or determined.

701.05 LAYING PIPE. Pipe laying shall begin at the downstream end of the line. The pipe shall be in contact with the foundation throughout its length. Bell or groove ends of pipe and outside circumferential laps of riveted metal pipe shall be placed facing upstream. Riveted seam metal pipe shall be placed with longitudinal laps at sides. Pipes in each continuous line shall have the same wall thickness. Metal pipes provided with lifting lugs shall be handled only by these lugs.
After pipe has been laid and before backfill is placed, the engineer will inspect the pipe for alignment, grade, integrity of joints, and coating damage.

701.06 JOINING PIPE.
   (a) Joint Usage:
      (1) Type 1 (T1) joints shall be used for side drains under drives and similar installations.
      (2) Type 2 (T2) joints shall be used for cross drains under roadways, including turnouts.
      (3) Type 3 (T3) joints shall be used for closed storm drain systems, flumes and siphons.
   (b) Concrete Pipe: Concrete pipe may be either bell and spigot, or tongue and groove. The method of joining pipe sections shall be such that ends are fully entered and inner surfaces are flush and even.
      An approved mechanical pipe puller shall be used for joining pipes over 36 inches (900 mm) in diameter. For pipe 36 inches (900 mm) or less in diameter, any approved method for joining pipe may be used which does not damage the pipe.
      Joints shall comply with Subsection 1006.05, and shall be sealed with gasket material installed in accordance with the manufacturer's recommendations.
   (c) Metal Pipe: Metal pipe shall be firmly joined by coupling bands. Bands shall be centered over the joint.
      For Type 1 joints, approved gasket material shall be placed in one corrugation recess on each side of the joint at the coupling band and on each band connection in such manner to prevent leakage.
      When Type 2 or 3 joints are specified, joining of metal pipe sections shall conform to the following provisions:
         (1) General: Band joints shall be sealed with gasket material. Gasket material shall be placed in accordance with the plan details.
         (2) Circular Section: Connecting bands shall be of an approved design and shall be installed in accordance with plan details.
         (3) Arch Section: Connecting bands shall be a minimum of 12 inches (300 mm) wide for pipe arch less than 36 inches (900 mm) round equivalent diameter, and a minimum of 21 inches (525 mm) wide for 36 inches (900 mm) round equivalent diameter pipe arch and greater. Bands shall be connected at the ends by approved angle or strap connections. Connecting bands used for 36 inches (900 mm) round equivalent diameter pipe arch and above shall be 2-piece bands.
   (d) Plastic Pipe: Joints for plastic pipe shall be either bell and spigot or split coupling bands.
      (1) Bell and Spigot Type Joint System: The method of joining pipe sections shall be such that ends are fully entered and inner surfaces are flush and even.
      Any approved method for joining pipe may be used which does not damage the pipe.
      Joints shall be approved and shall be sealed with a gasket system utilizing gasket material complying with Subsection 1006.06(a).
(2) Split Coupling Type Joint System: Split coupling bands shall comply with all dimensional and material requirements of Subsection 1006.07. The bands shall be centered over the joint. The split coupling band shall be secured to the pipe with a minimum of five stainless steel or other approved corrosion resistant bands.

Joints shall be approved and shall be sealed with gasket material. Gasket material shall be placed in the first two corrugation recesses on each side of the pipe connections. Gasket material shall also be placed on each band connection to prevent leakage. When flexible plastic gasket material is used it shall be a minimum of 1/2 inch (13 mm) in size. The bands shall be tightened to create overlap of the band and shall adequately compress the gasket material.

(e) Connections: Approved connections shall be used when joining new pipes to existing pipes. When concrete collars are required in order to extend the ends of existing pipes that have been damaged or to join different types or sizes of pipes, the concrete collars shall be constructed in accordance with plan details, the applicable requirements of Section 901, and as directed.

(f) Geotextile Fabric, Pipe Joints: For concrete, metal and plastic pipes, Types 2 and 3 joints shall be wrapped with geotextile fabric for a minimum of 12 inches (300 mm) on each side of joint for pipe 36 inches (900 mm) or less in diameter and a minimum of 18 inches (450 mm) on each side of the joint for pipe greater than 36 inches (900 mm) in diameter. Ends of the fabric shall be lapped at least 10 inches (250 mm). The edges and ends of fabric shall be suitably secured for the entire circumference of the pipe.

701.07 RELAYING PIPE. If specified or directed, existing pipes shall be removed and suitable sections relaid as specified for new pipes.

701.08 BACKFILLING.

(a) General: Prior to backfilling, pipes found to be damaged or out of alignment or grade shall be removed and reinstalled, or replaced.

Type A backfill material shall be stone, recycled portland cement concrete, flowable fill, or RAP.

Type B backfill materials are selected soils. Where Type B backfill materials are called for, Type A backfill materials may be substituted.

When corrugated metal pipe is used, the backfill material shall be tested and shall have a resistivity greater than 1500 ohm-cm and a pH greater than 5 when tested in accordance with DOTD TR 429 and DOTD TR 430 respectively.

When Type A backfill material is used, geotextile fabric surrounding this backfill shall be placed in accordance with Subsection 726.03 between the aggregate backfill material and all other natural or placed soils in the trench or embankment. Care shall be taken to prevent damage to geotextile fabric during placement of backfill material. For concrete pipe, the fabric shall enclose not only the initial backfill but shall be wrapped over the top of the pipe with at least 12 inches (300 mm) of overlap.

When a trench box or trench sheeting is used in unstable soils and/or for worker safety, and when moved during backfilling operations, filling and additional compaction of the disturbed zone of backfill must take place immediately and in a manner acceptable to the engineer.
Initial backfill is a structural backfill encasing the pipe from the bottom of the pipe to the springline for concrete pipe and to a point one foot (0.3 m) above the top of the pipe for both metal and plastic pipe. Final backfill is not a structural backfill and shall extend from the top of the initial backfill to the top of the natural ground or subgrade in cut areas or to the top of existing ground in fill areas. Any fill required above the final backfill is considered and treated as embankment.

(b) Backfill Applications: For projects using A+B+C bidding method where rigid and flexible pavement alternates are considered, backfill application (2) below, “Cross Drains Under Flexible Pavements”, shall apply for either rigid or flexible pavements.

1. Under Concrete Pavements: Type B backfill may be used as initial and final backfill for all pipes, culverts or drains under concrete pavements. Placement and compaction shall be as specified in Heading (d) below.

2. Cross Drains Under Flexible Pavements: All reaches, exclusive of those portions of the pipe which are under shoulders, of cross drains and all other culverts, pipes or drains that cross the centerlines of the new roadway or centerlines of existing roadways, such as intersections and are under flexible pavements shall receive an initial backfill of Type A material. Type B backfill materials may be used as final backfill for all pipes. Placement and compaction shall be as specified in Heading (c) and (d) below. Where the subgrade is above existing ground, embankment material as specified for the remainder of the project shall be used from the top of the final backfill to the top of the established embankment grade.

3. Other Drains Under Flexible Pavements: All reaches of all culverts, pipes or drains under flexible pavements that do not cross the centerlines of new roadway or centerlines of existing roadways, and exclusive of those portions of the pipe which are totally under shoulders, shall receive an initial and final backfill of Type B material. Placement and compaction shall be as specified in Heading (d) below. Where the subgrade is above existing ground, embankment material as specified for the remainder of the project shall be used from the top of the final backfill to the top of the established embankment grade.

4. Other Areas: All culverts, pipes or drains in nonpaved areas or paved areas that serve as driveways or shoulders shall receive an initial and final backfill of Type B material. Placement and compaction shall be as specified in Heading (d) below.

5. Pipes Subject to Construction Traffic; The embankment or pipe backfill shall be constructed to a minimum of 24 inches (600 mm) over the pipe before heavy construction equipment is allowed to cross the installation. Where practical, installations with less than 24 inches (600 mm) of cover over the top of the pipe shall be constructed after heavy hauling is completed over the pipe location. After completion of hauling operations, the contractor shall remove excess cover material. Pipe damaged by hauling and backfilling operations shall be removed and reinstated, or replaced, at no direct pay.

(c) Placement and Compaction; Type A Backfill: For all pipes, culverts and conduits under paved and nonpaved areas, where Type A backfill material is used, the Type A backfill shall be thoroughly hand compacted under the pipe haunches and then dynamically compacted in layers not exceeding 8 inches (200 mm) compacted thickness. Compaction under the haunches of the pipe shall initially be by hand tamping or other acceptable means, until a level is reached that the dynamic tamping can commence. Each lift shall be compacted by applying at least eight
passes of a hand operated, dynamic mechanical compaction device over the surface of each lift. With approval of the engineer, layer thickness may be increased to 12 inches (300 mm) with verification of satisfactory installation and performance. If flowable fill is used it shall be furnished, placed and consolidated in accordance with Section 710. The contractor shall control placement operations during initial backfill operations so as not to damage protective coatings on metal pipes. The contractor shall repair damaged coatings at no additional pay.

(d) Placement and Compaction; Type B Backfill: For all pipes, culverts and conduits, where Type B backfill is allowed, the Type B material shall be placed in layers not exceeding 8 inches (200 mm) compacted thickness. Compaction shall be with suitable mechanical equipment. With approval of the engineer, layer thickness may be increased to 12 inches (300 mm) with verification of satisfactory installation and performance.

(e) Placement and Compaction; Trenchless or Partial Trench Condition: All pipes, culverts, drains and conduits placed with any portion of the pipe above existing ground must also comply with Subsections (a), (b), (c) and (d) above for the portion of the pipe within a trench and that portion of the pipe not constructed in a trench. The width of initial and final backfill of that portion above existing ground and not within a trench will be constructed to such a width that the requirements for placement, compaction and density are met.

(f) Density Requirements: The in place density of Type A backfill materials and bedding materials, will not be measured or determined. Type A backfill, exclusive of RAP and flowable fill, shall be placed at or near optimum moisture content determined in accordance with DOTD TR 415 or 418. RAP materials shall be placed and compacted in a slightly moist condition.

The maximum dry density of initial or final Type B backfill under all paved areas which are to be under traffic will be determined in accordance with DOTD TR 415 or TR 418 and in-place density determined in accordance with DOTD TR 401. Initial and final Type B backfill under all paved areas, under traffic, shall be placed at or near optimum moisture content determined in accordance with DOTD TR 415 or TR 418. Each layer shall be compacted by approved methods prior to the placement of a subsequent layer. The engineer will approve the compaction method based upon validation that such method, including moisture control, will achieve at least 95 percent of maximum dry density as determined in accordance with DOTD TR 401. With approval of the engineer, density testing may be waived on subsequent layers with backfill installation in accordance with approved compaction methods and continued satisfactory performance.

Initial and final backfill in unpaved areas or paved areas such as shoulders or driveways, shall be placed evenly and compacted along the length of the culvert, pipe or drain from the top of the initial backfill to the top of the subgrade. Layered backfill shall be compacted at least to the density of the adjoining existing soils or the compaction required of the laterally adjoining layers of soil immediately outside the trench for embankment elevations. Initial and final backfill shall be placed and compacted at or near optimum moisture content determined in accordance with DOTD TR 415 or TR 418.

701.09 INSPECTION OF PIPES. After completion of embankment and prior to roadway surfacing, the engineer shall inspect pipes for proper alignment and integrity of joints. Any misaligned pipe or defective joints shall be corrected by the contractor at no direct pay.
(a) Plastic Pipe: Installed plastic pipe shall be tested to ensure that vertical deflections do not exceed 5.0 percent. Maximum allowable deflections shall be governed by the mandrel requirements stated herein.

Deflection tests shall be performed no sooner than 30 calendar days after installation and compaction of backfill. The pipe shall be cleaned and inspected for offsets and obstructions prior to testing.

For pipe 36 inches (900 mm) and less in diameter, a mandrel shall be pulled through the pipe by hand to ensure that maximum allowable deflections have not been exceeded. The mandrel shall be approved by the engineer prior to use. Use of an unapproved mandrel or a mandrel altered or modified after approval will invalidate the test. If the mandrel fails to pass, the pipe is overdeflected.

Unless otherwise permitted, overdeflected pipe shall be uncovered and, if not damaged, reinstalled. Damaged pipe shall not be reinstalled, but shall be removed and replaced with new pipe. Any pipe subjected to any method or process other than removal, which attempts, even successfully, to reduce or cure any overdeflection, shall be removed and replaced with new pipe.

The mandrel shall be a rigid, nonadjustable, odd-numbered legged (minimum 9 legs) mandrel having a length not less than its nominal diameter or 24 inches (600 mm), whichever is less. The minimum diameter at any point shall be 5.0 percent less than the base inside diameter of the pipe being tested. The mandrel shall be fabricated of steel, aluminum or other approved material fitted with pulling rings at each end. The nominal pipe size and outside diameter of the mandrel shall be stamped or engraved on some segment other than a runner. A suitable carrying case shall be furnished.

For pipe larger than 36 inches (900 mm) in diameter, deflection shall be determined by a method approved by the engineer. If a mandrel is selected, the minimum diameter, length, and other requirements shall conform to the above requirements.

Mandrel testing shall be conducted by the contractor in the presence of the engineer. Mandrel testing shall be at no direct pay.

(b) Metal Pipe: If the inside diameter of metal pipe or rise dimension of metal pipe arch deflects more than 5.0 percent from original dimensions, they shall be removed and reinstalled, unless they do not rebound or are damaged. Pipe or pipe arch which are damaged or do not rebound shall be removed and replaced at no direct pay. Measurement of deflection will be made by the engineer away from rerolled ends.

701.10 CLEANING PIPES.

(a) Existing Pipes: Pipes designated to be cleaned shall be cleaned of soil, debris and other materials to the invert of the pipe. Designated pipes shall be cleaned by approved methods that will not damage the pipes. Any damage caused by the contractor’s operations shall be satisfactorily repaired at no direct pay.

Removed soil, debris and other materials shall be disposed of in accordance with Subsection 202.02 or as otherwise approved in writing.

(b) Contractor Installed Pipes: Prior to final acceptance, pipes shall be cleaned of all debris and soil to the invert of the pipe at no direct pay.
Removed soil, debris and other materials shall be disposed of in accordance with Subsection 202.02 or as otherwise approved in writing.

701.11 STUBBING AND PLUGGING PIPES. When it is required that pipes be plugged, such plugs shall be constructed of Class R concrete complying with Section 901. Thickness of plug and method of construction shall be as directed.

When new pipes are to be stubbed into new or existing pipes or other structures, the connection shall be made with approved mortar complying with Subsection 702.02.

701.12 MEASUREMENT. Pipe, both new and relaid, will be measured in linear feet (lin m) as follows unless stated otherwise.

(a) Pipe not confined by fixed structures will be measured by the number of joints at the nominal length of each joint.

(b) Pipe confined by fixed structures will be measured along the pipe between the termini of pipe in structure walls.

(c) Pipe confined by a fixed structure on one end and unconfined at the other end will be measured along the pipe from the terminus of pipe in the structure wall to the unconfined end of pipe.

(d) Fabricating of pipe tees, elbows and other fittings will be measured per each fitting. The length of pipe in such fittings will be included in the pay length measurement of pipes of which they form a part.

(e) Excavation required for installation of pipes will not be measured for payment, except as otherwise specified in Subsection 203.14.

(f) Furnishing and placing backfill material below existing ground level for pipes will not be measured for payment. Backfill material needed to complete backfill above natural ground and around pipes that extend above natural ground will be measured and payment will be made under applicable earthwork items. When specified, flowable fill will be measured and paid for in accordance with Section 710.

(g) Plugging and stubbing of pipes will not be measured for payment.

(h) Cleaning existing pipes will be measured by the length of pipe cleaned and accepted.

(i) Concrete collars will be measured per each.

701.13 PAYMENT.

(a) Payment for pipe will be made at the contract unit price per linear foot (lin m) of the types and sizes specified.

When plastic pipe is specified on the plans or elected to be used by the contractor, payment will be made at the contract unit price per linear foot (lin m) of the types and sizes specified in accordance with the payment schedule of Table 701-1.
Table 701-1
Payment Schedule for Plastic Pipe

<table>
<thead>
<tr>
<th>Percent Payment</th>
<th>Stage of Completeness</th>
</tr>
</thead>
<tbody>
<tr>
<td>75</td>
<td>After placement and backfill has been completed</td>
</tr>
<tr>
<td>25</td>
<td>After the pipe has met vertical deflection requirements in accordance with Subsection 701.09(a)</td>
</tr>
</tbody>
</table>

(b) Payment for fabricating pipe tees, elbows and other fittings will be made at the contract unit price per each fitting.

(c) When unstable conditions are encountered, the additional excavation will not be measured for payment; however, the additional materials furnished and placed for the pipe foundation will be measured and paid for as follows:

1. Granular Materials: Payment will be made under the embankment item. The net section volume of the materials will be multiplied by 3 to determine the pay volume. When the contract does not include a pay item for embankment, payment will be made in accordance with Subsection 104.02.

2. Bedding Material: Measurement and payment will be made in accordance with Section 726. When the contract does not include a pay item for bedding material, payment will be made in accordance with Subsection 104.02.

(d) Payment for cleaning existing pipes will be made at the contract unit price per linear foot (lin m).

(e) Payment for concrete collars will be made at the contract unit price per each.

Payment will be made under:

<table>
<thead>
<tr>
<th>Item No.</th>
<th>Pay Item</th>
<th>Pay Unit</th>
</tr>
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<tbody>
<tr>
<td>701-01</td>
<td>Cross Drain Pipe (Size &amp; Type)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-02</td>
<td>Cross Drain Pipe Arch (Size &amp; Type)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-03</td>
<td>Storm Drain Pipe (Size &amp; Type)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-04</td>
<td>Storm Drain Pipe Arch (Size &amp; Type)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-05</td>
<td>Side Drain Pipe (Size)</td>
<td>Linear Foot (Lin m)</td>
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<tr>
<td>701-06</td>
<td>Side Drain Pipe Arch (Size)</td>
<td>Linear Foot (Lin m)</td>
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<tr>
<td>701-07</td>
<td>Yard Drain Pipe (Size)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-08</td>
<td>Relaying Pipe</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-09</td>
<td>Fabricating Pipe Fittings</td>
<td>Each</td>
</tr>
<tr>
<td>701-10</td>
<td>Reinforced Concrete Pipe (Extension)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-11</td>
<td>Reinforced Concrete Pipe Arch (Extension)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-12</td>
<td>Corrugated Metal Pipe (Extension)</td>
<td>Linear Foot (Lin m)</td>
</tr>
<tr>
<td>701-13</td>
<td>Corrugated Metal Pipe Arch (Extension)</td>
<td>Linear Foot (Lin m)</td>
</tr>
</tbody>
</table>
SECTION 704 – GUARD RAIL:
Subsection 704.03 – General Construction Requirements (01/05), Pages 368 and 369.
Add the following to Heading (d), Guard Rail End Treatments.
All end treatments shall bear a label indicating the manufacturer and exact product name of the end treatment along with its assigned NCHRP 350 test level. This label shall resist weathering and shall be permanently affixed to the railing in such a way as to be readily visible.

SECTION 706 – CONCRETE WALKS, DRIVES AND INCIDENTAL PAVING:
All Subsections within Section 706 (04/08), Pages 375 – 377.
Delete Section 706, Concrete Walks, Drives and Incidental Paving and substitute the following.

SECTION 706
CONCRETE WALKS, DRIVES AND INCIDENTAL PAVING

706.01 DESCRIPTION. This work consists of furnishing and constructing portland cement concrete walks, handicapped curb ramps, drives and incidental paving slabs in accordance with these specifications and in conformity with lines, grades and dimensions shown on the plans or established.

706.02 MATERIALS. Materials shall comply with the following Section or Subsections.

<table>
<thead>
<tr>
<th>Material</th>
<th>Subsection</th>
</tr>
</thead>
<tbody>
<tr>
<td>Portland Cement Concrete (Class M)</td>
<td>901</td>
</tr>
<tr>
<td>Joint Filler</td>
<td>1005.01(c)</td>
</tr>
<tr>
<td>Reinforcing Steel</td>
<td>1009.01</td>
</tr>
<tr>
<td>Curing Materials</td>
<td>1011.01</td>
</tr>
</tbody>
</table>

706.03 CONSTRUCTION REQUIREMENTS.
(a) Excavation: Excavation shall be made to required depth and width. The top of the subgrade shall be shaped and compacted to a firm, even surface conforming to the section shown on the plans. Unsuitable material shall be removed and disposed of in accordance with Subsection 202.02 and replaced with approved material at no direct pay.
(b) Forms: Forms shall be of wood or metal and shall extend the full depth of concrete. Forms shall be straight, clean and of sufficient strength to resist the pressure of concrete. Bracing of forms shall be such that forms remain in horizontal and vertical alignment until their removal.
Concrete may be placed by slip-form methods. Slip-formed concrete shall be placed with an approved machine designed to spread, vibrate, consolidate and finish concrete in one pass of the machine in such manner that minimum hand finishing is necessary. Sliding forms shall be
rigidly held together to prevent spreading of forms. After the passing of the side forms there shall be no noticeable slumping of concrete.

(c) Subgrade: The subgrade shall be thoroughly moistened immediately prior to placing concrete.

(d) Placing and Finishing: Concrete shall be placed on the subgrade, struck off to required thickness and tamped sufficiently to bring the mortar to the surface. The surface shall be finished with a wood float or steel trowel followed by brushing to a slightly rough finish. Joints and edges shall be rounded with an edging tool having a 1/4-inch (6 mm) radius.

(e) Joints:

(1) Expansion Joints: Expansion joints shall be filled with 1/2 inch (13 mm) thick preformed expansion joint filler. Expansion joints shall be installed at maximum 100-foot (30 m) intervals, and between intersecting paving and any fixed structure such as a building, bridge or curbing, and between intersecting paving and the handicapped curb ramps. Expansion joint material shall extend for the full width and depth of paving.

(2) Weakened Plane: Weakened planes shall be formed by a jointing tool or other acceptable means. Weakened planes shall extend into concrete for at least 1/4 of the depth and shall be approximately 1/8 inch (3 mm) wide.
   a. Walks: Spacing of weakened planes for walks shall be equal to the width of walk.
   b. Drives: A longitudinal weakened plane shall be formed along the centerline of drives more than 16 feet (5 m) wide, and transverse weakened planes shall be formed at not more than 16-foot (5 m) intervals.
   c. Incidental Paving: Weakened planes for incidental paving shall be formed at intervals not exceeding 30 times the thickness of the concrete in length or width. Incidental paving poured adjacent to jointed concrete shall be jointed to match existing joints, with intermediate joints formed as necessary not to exceed the maximum joint spacing.

(3) Construction Joints: Construction joints shall be formed around manholes, utility poles, etc., extending into paving and 1/4 inch (6 mm) thick preformed expansion joint filler shall be installed in these joints.

(4) Tie-ins: Tie-ins of existing concrete shall be made by full depth sawing at no direct pay.

(f) Curing: Concrete shall be cured in accordance with Subsection 601.10.

(g) Detectable Warning Surface for Handicap Ramps and At-Grade Sidewalk Intersections: Sidewalks, when intersecting with roadways, shall be equipped with a detectable warning surface system consisting of raised truncated domes as a transition between the sidewalk and the street as required by the Americans with Disabilities Act, 28 CFR Part 36, ADA Standards for Accessible Design.

Detectable warnings (truncated domes) shall be installed on the ramp surface over the full width of the ramp throat for a distance of 24 inches (600 mm) in the direction of travel from the back of the curb. Detectable warnings (truncated domes) shall also be installed on at-grade sidewalks intersecting with roadways for a distance of 36 inches (900 mm) in the direction of travel from the end of the sidewalk. Truncated domes shall be laid out on a square grid in order to allow enough space for wheelchairs to roll between the domes.
Light reflectance of the truncated domes and the underlying surface must meet the 70 percent contrast requirement of ADAAG.

706.04 MEASUREMENT. Quantities of concrete walks, drives and incidental paving slabs for payment will be the design quantities as specified on the plans and adjustments thereto. Design quantities will be adjusted if the engineer makes changes to adjust to field conditions, if design errors are proven or if design changes are made. Design areas are based on the horizontal dimensions shown on the plans. Excavation, backfill, reinforcing steel and joint materials will not be measured for payment.

Handicapped curb ramps, including the detectable surface warning system, will be measured per each.

Detectable surface warning systems for at-grade sidewalk intersection will not be measured for payment.

706.05 PAYMENT. Payment for concrete walks, drives and incidental paving will be made on a lot basis at the contract unit price per square yard (sq m), adjusted in accordance with the following provisions. Payment for each lot will be made in accordance with Table 901-6. Size, sampling, and testing of each concrete lot shall be in accordance with the Materials Sampling Manual.

Payment for handicapped curb ramps, including the detectable surface warning system, will be made by each and shall include, but not limited to, curb transitions, detectable warning system, gutter, landing and base.

Payment will be made under:

<table>
<thead>
<tr>
<th>Item No.</th>
<th>Pay Item</th>
<th>Pay Unit</th>
</tr>
</thead>
<tbody>
<tr>
<td>706-01</td>
<td>Concrete Walk (    inch (mm) Thick)</td>
<td>Square Yard (Sq m)</td>
</tr>
<tr>
<td>706-02</td>
<td>Concrete Drive (    inch (mm) Thick)</td>
<td>Square Yard (Sq m)</td>
</tr>
<tr>
<td>706-03</td>
<td>Incidental Concrete Paving</td>
<td></td>
</tr>
<tr>
<td></td>
<td>(    inch (mm) Thick)</td>
<td>Square Yard (Sq m)</td>
</tr>
<tr>
<td>706-04</td>
<td>Handicapped Curb Ramps</td>
<td>Each</td>
</tr>
</tbody>
</table>

SECTION 713 – TEMPORARY TRAFFIC CONTROL:
Subsection 713.06 – Pavement Markings (08/06), Pages 400 – 403.

Delete Table 713-1, Temporary Pavement Markings and substitute the following.
## Table 713-1
### Temporary Pavement Markings\(^1,2\)

<table>
<thead>
<tr>
<th></th>
<th>Two-lane Highways</th>
<th>Undivided Multilane Highways</th>
<th>Divided Multilane Highways</th>
</tr>
</thead>
<tbody>
<tr>
<td><strong>SHORT TERM</strong></td>
<td>ADT&lt;1500; or ADT&gt;1500 and time&lt;3 days</td>
<td>Lane lines 4-foot (1.2 m) tape on 40-foot (12 m) centers; with &quot;Do Not Pass&quot; and &quot;Pass With Care&quot; signs as required</td>
<td></td>
</tr>
<tr>
<td></td>
<td>ADT&gt;1500; Time&gt;3 days and&lt;2 weeks</td>
<td>Lane lines 4-foot (1.2-m) tape on 40-foot (12-m) centers with no passing zone markings</td>
<td></td>
</tr>
<tr>
<td><strong>ALL ADT's</strong></td>
<td>with time&lt;2 weeks</td>
<td>Lane lines 4-foot (1.2m) tape on 40-foot (12 m) centers; double yellow centerline</td>
<td>Lane lines 4-foot (1.2 m) tape on 40-foot (12 m) centers</td>
</tr>
<tr>
<td><strong>LONG TERM</strong></td>
<td>All ADT's with time &gt;2 weeks</td>
<td>Standard lane lines, no-passing zone markings, legends and symbols and when pavement width is 22 feet (6.7 m) or greater, edge lines</td>
<td>Standard lane lines, centerlines, edge lines, and legends and symbols.</td>
</tr>
<tr>
<td></td>
<td></td>
<td>Standard lane lines, centerlines, edge lines, and legends and symbols.</td>
<td>Standard lane lines, centerlines, edge lines, and legends and symbols.</td>
</tr>
</tbody>
</table>

\(^1\)No-passing zones shall be delineated as indicated whenever a project is open to traffic.

\(^2\)On all Asphaltic Surface Treatments that are open to traffic and used as a final wearing course or as an interlayer, temporary pavement markings (tabs) on 20-foot (6 m) centers shall be used, in lieu of the 4-foot (1.2 m) tape, on 40-foot (12 m) centers.

### SECTION 729 – TRAFFIC SIGNS AND DEVICES:
Subsection 729.02 – Materials (04/08), Pages 456 and 457.

Delete the contents of Heading (a), Sign and Marker Sheeting, and substitute the following.

(a) Sign and Marker Sheeting: Sheeting material for sign panels, delineators, barricades and other markers shall comply with Section 1015. All permanent signs shall meet the requirements of ASTM D 4956, Type X.
Subsection 729.04, Fabrication of Sign Panels and Markers (04/08), Pages 458 – 460.

Delete the third paragraph of Heading (c), Sheeting Application and substitute the following.

ASTM D 4956 Type X reflective sheeting shall be applied with an orientation determined by the engineer to obtain the optimum entrance angle performance. Fabricated vertical splices in ASTM D 4956 Type X reflective sheeting will be allowed only when the horizontal dimension of the sign face or attached shield is in excess of the maximum manufactured width of the sheeting. Fabricated vertical splices in ASTM D 4956 Type X reflective sheeting will also be allowed when the specified orientation will create excessive sheeting waste.

SECTION 804 – DRIVEN PILES:
Subsection 804.08 – Construction Requirements (04/07), Pages 548 – 554.

Delete the first sentence of Heading (a), Preboring and substitute the following.

Preboring by augering, wet-rotary drilling, or other methods used to facilitate pile driving will not be permitted unless specified in the plans or allowed by the engineer.

Delete the first sentence of Heading (b), Jetting and substitute the following.

Jetting will not be permitted unless allowed in the plans or allowed by the engineer.

SECTION 901 – PORTLAND CEMENT CONCRETE:
Subsection 901.06 – Quality Control of Concrete (08/06), Pages 726 – 731.

Add the following to the contents of Heading (b), Quality Control Tests.

The contractor shall be responsible for monitoring the components (cement, mineral and chemical admixtures, aggregates) in their mix to protect against any changes due to component variations. As component shipments arrive, the contractor shall verify slump, air content and set time by testing at ambient temperatures. The contractor shall make adjustments to the mix design to rectify any changes which would adversely affect constructability, concrete placement or the specifications. The contractor shall submit test results to the Department for review each day of paving. Testing to validate component consistency will be documented on the control logs. Conformance or variation in mix parameters (workability, set times, air content, etc.) shall be noted on the control logs. The contractor shall provide a copy of the proposed testing plan to the engineer for record. Acceptance of the plan does not relieve the contractor’s responsibility for consistency.

Subsection 901.08 – Composition of Concrete (12/05), Pages 732 – 734.

Add the following to Heading (a).

The blended cement containing up to 50 percent of grade 100 or grade 120 ground granulated blast-furnace slag must be in compliance with Subsection 1001.04 for portland blast-furnace slag cement.
SECTION 1001 – HYDRAULIC CEMENT:

Delete the contents of this subsection and substitute the following.

1001.01 PORTLAND CEMENT. Portland cement shall be from an approved source listed in QPL 7 and shall comply with AASHTO M 85.

Alkali content calculated as sodium oxide equivalent shall not exceed 0.60 percent by weight for all types of cement.

SECTION 1003 – AGGREGATES:
Subsection 1003.02 – Aggregates for Portland Cement Concrete and Mortar (07/07), Pages 763 – 766.

Delete the contents of Heading (c), Aggregates for Types B and D Pavements, and substitute the following.

(c) Aggregates for Types B and D Pavements: For the combined aggregates for the proposed portland cement concrete pavement mix, the percent retained based on the dry weight (mass) of the total aggregates shall meet the requirements of Table 1003-1A for the type of pavement specified in the plans. Additionally, the sum of the percents retained on any two adjacent sieves so designated in the table shall be at least 12 percent of the total combined aggregates. The maximum amounts by weight (mass) of deleterious materials for the total aggregate shall be the same as shown in Subsection 1003.02(b).
Table 1003-1A
Aggregates for Types B and D Pavements

<table>
<thead>
<tr>
<th>U.S. Sieve</th>
<th>Metric Sieve</th>
<th>Percent Retained of Total Combined Aggregates</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td></td>
<td>Pavement Type</td>
</tr>
<tr>
<td></td>
<td></td>
<td>Type B</td>
</tr>
<tr>
<td>2 1/2 inch</td>
<td>63 mm</td>
<td>0</td>
</tr>
<tr>
<td>2 inch</td>
<td>50 mm</td>
<td>0</td>
</tr>
<tr>
<td>1 1/2 inch</td>
<td>37.5 mm</td>
<td>0-20</td>
</tr>
<tr>
<td>1 inch</td>
<td>25.0 mm</td>
<td>0-20</td>
</tr>
<tr>
<td>3/4 inch</td>
<td>19.0 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>1/2 inch</td>
<td>12.5 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>3/8 inch</td>
<td>9.5 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>No. 4</td>
<td>4.75 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>No. 8</td>
<td>2.36 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>No. 16</td>
<td>1.18 mm</td>
<td>5-20</td>
</tr>
<tr>
<td>No. 30</td>
<td>600 μm</td>
<td>5-20</td>
</tr>
<tr>
<td>No. 50</td>
<td>300 μm</td>
<td>0-20</td>
</tr>
<tr>
<td>No. 100</td>
<td>150 μm</td>
<td>0-20</td>
</tr>
<tr>
<td>No. 200</td>
<td>75 μm</td>
<td>0-5</td>
</tr>
</tbody>
</table>

Note: For the sieves in the shaded areas, the sum of any two adjacent sieves shall be a minimum of 12 percent of the total combined aggregates.

Each type of aggregate to be used in the proposed mixture shall be sampled and tested individually. The percent of total combined aggregates retained shall be determined mathematically based on the proportions of the combined aggregate blend. All gradation calculations shall be based on percent of dry weight (mass).

SECTION 1005 – JOINT MATERIALS FOR PAVEMENTS AND STRUCTURES:
Subsection 1005.04 – Combination Joint Former/Sealer (11/05), Pages 782 and 783.

Delete Heading (a) and substitute the following.

(a) Description: This joint former/sealer is intended for use in simultaneously forming and sealing a weakened plane in portland cement concrete pavements.

The material shall consist of an elastomeric strip permanently bonded either mechanically or chemically at the top of each of two rigid plastic side frames and covered with a removable plastic top cap. Side frames shall be of such configuration that when the sealer is inserted into plastic concrete and vibrated, a permanent bond forms between side frames and concrete.

Delete Heading (b)(1) and substitute the following.

(1) Elastomer: The elastomer strip portion of the material shall be manufactured from vulcanized elastomeric compound using polymerized chloroprene or thermoplastic vulcanizate as the base polymer, and shall comply with the following requirements:
### Property Specifications

<table>
<thead>
<tr>
<th>Property</th>
<th>ASTM Test Method</th>
<th>Requirements</th>
</tr>
</thead>
<tbody>
<tr>
<td>Plasticized Chloroprene</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Tensile Strength, kPa, Min.</td>
<td>D 412</td>
<td>12,400 7,400</td>
</tr>
<tr>
<td>Elongation at Break, % Min.</td>
<td>D 412</td>
<td>200 400</td>
</tr>
<tr>
<td>Hardness, Shore A</td>
<td>D 2240</td>
<td>65 ± 10 65 ± 10</td>
</tr>
<tr>
<td>Properties after Aging, 70 h @ 100°C</td>
<td>D 573</td>
<td></td>
</tr>
<tr>
<td>Tensile Strength, % Loss, Max.</td>
<td></td>
<td>20 20</td>
</tr>
<tr>
<td>Elongation, % loss, Max.</td>
<td></td>
<td>25 25</td>
</tr>
<tr>
<td>Hardness, pts. increase, Max.</td>
<td></td>
<td>10 10</td>
</tr>
<tr>
<td>Ozone Resistance, 20% strain or bentloop,</td>
<td>D 1149</td>
<td>no cracks no cracks</td>
</tr>
<tr>
<td>300 pphm in air, 70 h @ 40°C</td>
<td></td>
<td></td>
</tr>
<tr>
<td>Oil Swell, IRM 903, 70 h @ 100°C</td>
<td>D 471</td>
<td>45 75</td>
</tr>
</tbody>
</table>

Delete Headings (b)(2) and (b)(3) and substitute the following:

2. Bond of Elastomer to Plastic: The force required to shear the elastomer from the plastic shall be a minimum of 5.0 pounds per linear inch (90 g/mm) of sealer when tested in accordance with DOTD TR 636.

3. Bond of Plastic to Cement Mortar: This bond will be evaluated and shall meet the following requirements:

   The force required to separate the cement mortar from the plastic shall be a minimum of 5.0 pounds per linear inch (90 g/mm) of sealer when tested in accordance with DOTD TR 636.

### Section 1006 – Concrete and Plastic Pipe:

Subsection 1006.09 – Plastic Yard Drain Pipe (06/07), Page 789.

Delete the contents of Subheading (a)(3), Ribbed Polyvinyl Chloride Pipe (RPVCP) and substitute the following.

Ribbed Polyvinyl Chloride Pipe (RPVCP): Ribbed Polyvinyl Chloride Pipe shall comply with ASTM F 794, Series 46 or ASTM F 949 (46 psi).

### Section 1013 – Metals:

Subsection 1013.09 – Steel Piles (08/06) Page 822.

Delete the title and references to “Steel Piles” in this subsection and substitute “Steel H Piles”.

### Section 1015 – Signs and Pavement Markings:

Subsection 1015.04 – Sign Panels (05/07), Pages 832 and 833.

Delete the contents of Heading (a), Permanent Sign Panels and substitute the following.
(a) Permanent Sign Panels: Flat panels shall be aluminum sheets or plates complying with ASTM B 209, Alloy 6061-T6 or Alloy 5052-H38. Extruded aluminum panels shall comply with ASTM B 221 (ASTM B 221M), Alloy 6063-T6 and after fabrication, have a flatness equal to or less than 0.031 inch per foot of length and 0.004 inch per inch of width.

Subsection 1015.05 - Reflective Sheeting (04/08), Pages 833 – 838.
Delete the contents of this subsection and substitute the following.

1015.05 REFLECTIVE SHEETING.
(a) Permanent and Temporary Standard Sheeting: Reflective sheeting shall be one of the following standard types as specified on the plans and complying with ASTM D 4956 except as modified herein. Permanent warning, regulatory, guide and supplemental guide sign sheeting shall meet the requirements of ASTM D 4956 Type X. Reflective sheeting for temporary signs and devices shall meet the requirements of ASTM D 4956 Type III except as noted in Subsection 1015.05(f). Reflective sheeting shall be an approved product listed in QPL 13.

Type III - A high-intensity retroreflective sheeting that is typically encapsulated glass-bead retroreflective material.

Type VI - An elastomeric high-intensity retroreflective sheeting without adhesive. This sheeting is typically a vinyl microprismatic retroreflective material.

Type X - A super high-intensity retroreflective sheeting having highest retroreflectivity characteristics at medium distances. This sheeting is typically an unmetalized microprismatic retroreflective element material.

(b) Fluorescent Pink Retroreflective Sheeting: Signs for temporary control of traffic through incident management areas shall be Type VI fluorescent pink retroreflective sheeting and shall comply with the MUTCD. Temporary traffic control signs for incident management shall be placed to notify motorists of upcoming incidents on the roadway, and shall be removed from public view once the incident has been managed. Physical properties shall comply with ASTM D 4956. Photometric properties shall be as follows.

(1) Retroreflectivity: Minimum Coefficients of Retroreflection shall be as specified in Table 1015-1.
Table 1015-1
Coefficients of Retroreflection for Fluorescent Pink Sheeting\(^1\)

<table>
<thead>
<tr>
<th>Observation Angle, degrees</th>
<th>Entrance Angle, degrees</th>
<th>Fluorescent Pink</th>
</tr>
</thead>
<tbody>
<tr>
<td>0.2</td>
<td>-4</td>
<td>100</td>
</tr>
<tr>
<td>0.2</td>
<td>+30</td>
<td>40</td>
</tr>
<tr>
<td>0.5</td>
<td>-4</td>
<td>40</td>
</tr>
<tr>
<td>0.5</td>
<td>+30</td>
<td>15</td>
</tr>
</tbody>
</table>

\(^1\)Minimum Coefficient of Retroreflection (R\(_A\)) (cd lx\(^{-1}\)m\(^{-2}\))

(2) Color and Daytime Luminance: Color Chromaticity Coordinates and Daytime Luminance Factors shall be as specified in Table 1015-2.

Table 1015-2
Fluorescent Pink Color Specifications Limits (Daytime)

<table>
<thead>
<tr>
<th>Chromaticity Coordinates (corner points) (^1)</th>
<th>Luminance Factor, min.</th>
</tr>
</thead>
<tbody>
<tr>
<td>1 x = 0.450, y = 0.270</td>
<td>Y% = 25</td>
</tr>
<tr>
<td>2 x = 0.590, y = 0.350</td>
<td></td>
</tr>
<tr>
<td>3 x = 0.644, y = 0.290</td>
<td></td>
</tr>
<tr>
<td>4 x = 0.536, y = 0.230</td>
<td></td>
</tr>
</tbody>
</table>

\(^1\)The four pairs of chromaticity coordinates measured with CIE 2\(^{°}\) Standard Observer and 45/0 (0/45) geometry and CIE D65 Standard Illuminant.

(c) Adhesive Classes: The adhesive required for retroreflective sheeting shall be Class 1 (pressure sensitive) as specified in ASTM D 4956.

(d) Accelerated Weathering: Reflective sheeting, when processed, applied and cleaned in accordance with the manufacturer's recommendations shall perform in accordance with the accelerated weathering standards in Table 1015-3.
### Table 1015-3
**Accelerated Weathering Standards**

<table>
<thead>
<tr>
<th>Type</th>
<th>Retroreflectivity</th>
<th>Colorfastness</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Orange/Fluorescent Orange</td>
<td>All colors, except orange/Fluorescent Orange</td>
</tr>
<tr>
<td>III</td>
<td>1 year 80⁴</td>
<td>3 years 80⁴</td>
</tr>
<tr>
<td>III (for drums)</td>
<td>1 year 80⁴</td>
<td>1 year 80⁴</td>
</tr>
<tr>
<td>VI</td>
<td>1/2 year 50⁵</td>
<td>1/2 year 50⁵</td>
</tr>
<tr>
<td>X</td>
<td>1 year 80⁶</td>
<td>3 years 80⁶</td>
</tr>
</tbody>
</table>

1. At an angle of 45° from the horizontal and facing south in accordance with ASTM G 7 at an approved test facility in Louisiana or South Florida.
2. Percent retained retroreflectivity of referenced table after the outdoor test exposure time specified.
3. Colors shall conform to the color specification limits of ASTM D 4956 after the outdoor test exposure time specified.
4. ASTM D 4956, Table 8.
5. ASTM D 4956, Table 13.
6. ASTM D 4956, Table 4.

(e) Expected Sign Life Data and Performance: The sheeting manufacturer shall supply expected retroreflectivity service life curves for each of the following sign sheeting colors: white, green, blue, brown, red, and yellow. The service life curves shall be plots of the 95 percent expected life plotted on an x-y graph with life years on the x-axis and retroreflectivity on the y-axis. The expected life shall account for worst case installations, equivalent to an installation in South Louisiana with the sign facing to the South. The sheeting manufacturer shall also supply a table of expected life values taken from the service life curves for Revision Number 2 to the 2003 Edition of the MUTCD minimum reflectivity requirements published in the Federal Register on December 21, 2007. Reflective sheeting for signs, when processed, applied and cleaned in accordance with the manufacturer's recommendations shall perform outdoors in accordance with the performance standards in Table 1015-4.
Table 1015-4
Reflective Sheeting Performance Standards

<table>
<thead>
<tr>
<th>Type</th>
<th>Retroreflectivity(^1) -- Durability(^2)</th>
<th>Colorfastness(^3)</th>
</tr>
</thead>
<tbody>
<tr>
<td></td>
<td>Orange/Fluorescent Orange</td>
<td>All colors, except orange/Fluorescent Orange</td>
</tr>
<tr>
<td>III</td>
<td>3 years 80(^4)</td>
<td>10 years 80(^4)</td>
</tr>
<tr>
<td>X</td>
<td>3 years 80(^5)</td>
<td>7 years 80(^5)</td>
</tr>
</tbody>
</table>

\(^1\)Percent retained retroreflectivity of referenced table after installation and the field exposure time specified.

\(^2\)All sheeting shall maintain its structural integrity, adhesion and functionality after installation and the field exposure time specified.

\(^3\)All colors shall conform to the color specification limits of ASTM D 4956 after installation and the field exposure time specified.

\(^4\)ASTM D4956, Table 8.

\(^5\)ASTM D 4956, Table 4.

(f) Temporary Signs, Barricades, Channelizing Devices, Drums and Cones: Reflective sheeting for temporary signs, barricades and channelizing devices, shall meet the requirements of ASTM D 4956, Type III except that temporary warning construction signs used on the mainline of freeways and expressways shall be fluorescent orange and meet the requirements of ASTM D 4956, Type X.

Reflective sheeting for vertical panels shall meet the requirements of ASTM D 4956, Type III.

Reflective sheeting for drums shall be a minimum of 6 inches (150 mm) wide and shall meet the requirements of ASTM D 4956, Type III, and the Supplementary Requirement S2 for Reboundable Sheeting as specified in ASTM D 4956. Reflective sheeting for traffic cone collars shall meet the requirements of ASTM D 4956, Type III or Type VI.

(g) Sheeting Guaranty. The contractor shall provide the Department with a guaranty from the sheeting manufacturer stating that if the retroreflective sheeting fails to comply with the performance requirements of this subsection, the sheeting manufacturer shall do the following:
Table 1015-5
Manufacturer's Guaranty-Reflective Sheeting

<table>
<thead>
<tr>
<th>Type</th>
<th>Manufacturer shall restore the sign face in its field location to its original effectiveness at no cost to the Department if failure occurs during the time period as specified below</th>
<th>Manufacturer shall replace the sheeting required to restore the sign face to its original effectiveness at no cost to the Department if failure occurs during the time period as specified below</th>
</tr>
</thead>
<tbody>
<tr>
<td>Orange/Fluorescent Orange</td>
<td>All colors, except orange/Fluorescent Orange</td>
<td>All colors, except orange/Fluorescent Orange</td>
</tr>
<tr>
<td>III</td>
<td>&lt;3 years</td>
<td>&lt;7 years</td>
</tr>
<tr>
<td>X</td>
<td>&lt;3 years</td>
<td>&lt;5 years</td>
</tr>
</tbody>
</table>

1 From the date of sign installation.

Replacement sheeting for sign faces, material, and labor shall carry the unexpired guaranty of the sheeting for which it replaces.

The sign fabricator shall be responsible for dating all signs with the month and year of fabrication at the time of sign fabrication. This date shall constitute the start of the guaranty obligation period.

Subsection 1015.11 - Preformed Plastic Pavement Marking Tape (06/07), Pages 842 – 844.

Delete the contents of this subsection and substitute the following.

1015.11 PREFORMED PLASTIC PAVEMENT MARKING TAPE.

(a) General: Preformed plastic pavement marking tape shall be approved products listed on QPL 64 and shall comply with ASTM D4505 Retroreflectivity Level I or Level II, or DOTD Intersection Grade (as specified below), except as modified herein. The marking tape shall be Class 2 or 3. The type and color shall be in accordance with the plans and the MUTCD.

(b) Thickness: All preformed plastic pavement marking tape shall have a minimum overall thickness of 0.060 inches (1.5 mm) when tested without the adhesive.

(c) Friction Resistance: The surface of the Retroreflectivity Level II preformed plastic pavement marking tape shall provide a minimum frictional resistance value of 35 British Polish Number (BPN) when tested according to ASTM E303. The surface of the Retroreflectivity Level I and DOTD Intersection Grade preformed plastic pavement marking tape shall provide a minimum frictional resistance value of 45 BPN when tested according to ASTM E303. Values for the Retroreflectivity Level I material with a raised surface pattern as defined in ASTM D4505 are calculated by averaging values taken at downweb and at a 45 degrees angle from downweb.
(d) Retroreflective Requirements: The preformed plastic pavement marking tape shall have the minimum initial specific luminance values shown in Table 1015-7 when measured in accordance with ASTM D 4061.

<table>
<thead>
<tr>
<th>Type</th>
<th>Observation Angle, degrees</th>
<th>Entrance Angle, degrees</th>
<th>Specific Luminance (mcd/sq m/lx)</th>
</tr>
</thead>
<tbody>
<tr>
<td>Retroreflectivity Level I</td>
<td>1.05</td>
<td>88.76</td>
<td>White 500 Yellow 300</td>
</tr>
<tr>
<td>DOTD Intersection Grade</td>
<td>1.05</td>
<td>88.76</td>
<td>White 375 Yellow 250</td>
</tr>
<tr>
<td>Retroreflectivity Level II</td>
<td>1.05</td>
<td>88.76</td>
<td>White 250 Yellow 175</td>
</tr>
</tbody>
</table>

(e) Durability Requirements: The DOTD Intersection Grade preformed plastic pavement marking tape shall show no appreciable fading, lifting or shrinkage for at least 12 months after placement when placed in accordance with the manufacturer's recommended procedures on pavement surfaces having a daily traffic count not to exceed 15,000 ADT per lane.

The Retroreflectivity Level I preformed plastic pavement marking tape shall show no appreciable fading, lifting or shrinkage for at least 4 years after placement for longitudinal lines and at least 2 years after placement for symbols and legends.

The Retroreflectivity Level I preformed plastic pavement marking tape shall also retain the following reflectance values for the time period detailed in Table 1015-8.

<table>
<thead>
<tr>
<th>Time</th>
<th>Observation Angle, degrees</th>
<th>Entrance Angle, degrees</th>
<th>Specific Luminance (mcd/sq m/lx)</th>
</tr>
</thead>
<tbody>
<tr>
<td>1 year</td>
<td>1.05</td>
<td>88.76</td>
<td>White 400 Yellow 240</td>
</tr>
<tr>
<td>4 years (2 years for symbols and legend)</td>
<td>1.05</td>
<td>88.76</td>
<td>White 100 Yellow 100</td>
</tr>
</tbody>
</table>

(f) Plastic Pavement Marking Tape Guaranty (DOTD Intersection Grade and Retroreflectivity Level I): If the plastic pavement marking tape fails to comply with the performance and durability requirements of this subsection within 12 months for DOTD Intersection Grade and 4 years for Retroreflectivity Level I, the manufacturer shall replace the plastic pavement marking material at no cost to the Department.

SECTION 1020 – TRAFFIC SIGNALS:
Subsection 1020.01 – Traffic Signal Heads (06/07), Pages 873 – 884.

Delete the contents of Heading (a), General Requirements and substitute the following.
(a) General Requirements: Traffic signal sections, beacon sections and pedestrian signal sections shall be of the adjustable type. Materials and construction of each section shall be the same.

Signals shall be constructed for either 8 or 12-inch (200 mm or 300 mm) lens in accordance with the plans. Signal sections shall have three to five sections per face and beacon sections have only one section per face. Signal sections and associated brackets shall be finished inside and out with two coats of high grade dark olive green enamel, color number 14056 according to Federal Standard No. 595b with each coat independently baked. Visors shall be coated green on the outside and black on the inside. Edges shall be deburred and smooth with no sharp edges.

Delete the sixth paragraph of Heading (a), Pedestal Support Signal Poles, and substitute the following.

Pedestals shall be finished with at least one coat of rustproofing primer, applied to a clean surface and one coat of dark olive green enamel, color number 14056 according to Federal Standard No. 595b.
TEMPORARY SIGNS & BARRICADES (ITEM 713-01):
The contractor is responsible for procuring and setting up all detour signs for this project. Refer to Plan-Galliano Bridge.dgn sheet on page E-45 for sign details and sign placement location.

MOBILIZATION (ITEM 727-01):
The contractor cannot mobilize to the site until having approval from the project engineer to do so. The contractor must mobilize to the site two (2) times. The first time is to remove the primary gear reducer’s gear train and one (1) secondary gear reducer’s gear train for the purpose of reverse engineering the gear boxes. Once the gear trains have been reversed engineered, the contractor must reinstall the old gear trains until the new gear trains are completely fabbed. During the fabrication time of the gear trains, the bridge must be left in a full functional state. After the gear trains are fabbed, the contractor shall mobilize again to take all three (3) gearboxes down for rehabilitation. During this phase, the bridge must be left open to marine traffic.

If the contractor decides to exercise the gearbox replacement options of all three (3) gearboxes, then the contractor cannot mobilize until the new gearboxes are ready for installation.

REMOVAL AND INSTALLATION OF GEARBOXES (ITEM S-001):
The contractor shall perform all appropriate preparatory work on the bridge necessary for safe removal and reinstallation of the one (1) primary, parallel shaft, gear reducer and two (2) secondary, right-angle, gear reducers. The contractor is responsible for ensuring the movable span and mechanical systems are secure while the gear reducers are removed from the structure. The contractor must provide the project engineer and project manager with details as to how the bridge will be secured for approval.

The contractor shall remove all three (3) gear reducers from the site and transport to a gear reducer manufacturer that is pre-approved by the project manager. See the “General Mechanical Notes” for suggested reducer manufacturers.

REHABILITATION OF ONE (1) PRIMARY & TWO (2) SECONDARY GEAR REDUCERS (ITEM S-002):
After removal of all three (3) gear reducers from the structure, the contractor has two (2) options:

1. Replace ALL internal components from all three (3) gear reducers:
The contractor shall reverse-engineer and replace all internal gear reducer components (gears, differential, shafts, clutch, bearings, seals, etc.) as per original (as-built) specifications. The clutch linkage must be compatible with external clutch control system. Where previous AGMA Standards are outdated, the contractor must use current AGMA standards for gear reducer components. The interior and exterior of the housing shall be cleaned, sandblasted, and painted as per the paint specifications in the “General Mechanical Notes” section. It is the contractor’s responsibility to ensure the new gear reducer components are fabricated in accordance with current AGMA standards as well as the specifications in the “General Mechanical Notes” section.

2. Replace all three (3) gear reducers COMPLETELY:
The contractor shall replace all gear reducers (including housing), entirely, with new gear reducers which are compatible with current operating machinery (gear ratio, speed & horsepower rating, clutch controls, base mount, etc.). It is the contractor’s responsibility
to ensure the new gear reducers are fabricated in accordance with current AGMA standards as well as the specifications in the “General Mechanical Notes” section.

The contractor is wholly responsible for the integration of the gear reducers into the existing mechanical system. All gear reducers shall be refurbished/replaced in accordance with the specifications in the “General Mechanical Notes” section.

If modifications to the clutch linkages are made such that the clutch control systems must be modified, this work will be performed by the contractor at no additional cost to the LA DOTD.

After gear reducers are refurbished/replaced, and testing is complete, the contractor shall install the gearboxes on the structure. During installation, at least one representative from the gear reducer manufacturer shall be present. The contractor is responsible for ensuring the refurbished gear reducers operate appropriately and are compatible with existing mechanical system of structure. The contractor must provide LA DOTD charts that show lead, profile, and gear spacing.

MEASUREMENT AND PAYMENT: Quantities shall be measured on a lump sum basis, which includes all materials, equipment, and labor necessary for the Rehabilitation of One (1) Primary & Two (2) Secondary Gear Reducers.

REHABILITATION OF DRIVELINE SYSTEM (ITEM S-003):
The contractor shall replace all pillow block bearings on the parallel and transverse drive lines. There are six (6) pillow block bearings on the parallel driveline and four (4) pillow block bearings on the transverse driveline. It is the contractor’s responsibility for all dimensions when selecting the pillow block bearings.

The contractor shall inspect all couplings to ensure proper operation of the driveline. If any coupling is determined in need of replacement, a change order shall be issued.

The contractor shall laser-aligned the drive lines after the gearboxes are installed to insure proper function and longevity of the gearboxes.
STATE PROJECT NO. 407-01-0046
OTHER SPECIFICATIONS

All work shall be performed in accordance with the 2006 Edition of the “Louisiana Standard Specifications for Roads & Bridges” and the 2007 Edition of the “AASHTO Standard Specifications for Movable Highway Bridges” unless otherwise specified herein, or in the plans or specifications. The specifications covered in the “General Mechanical Notes” are intended to govern general situations not specifically covered in the plans or specifications. The contractor shall consider every item in this section to be followed by the phrase “unless otherwise specified in the plans or specifications.”

The contractor shall provide all material, equipment, tools, labor, and expertise to purchase, fabricate, transport, install/erect, align/adjust, paint, lubricate, rehabilitate/repair, and test all the mechanical systems described in the plans and specifications. The contractor is expected to have sufficient expertise to interpret the plans and specifications such that all repairs are performed in accordance with the intentions of plans and specifications. If any portion of the mechanical work is not clearly or accurately defined in the plans or specifications, the contractor shall ask the project engineer for clarification before proceeding with the work.

SHOP WORK

All parts and full assemblies shall be stored indoors and protected from the weather while at the shop.

All finished mating surfaces shall have a 125 finish and coated with NO-OX-ID “a special” grease as manufactured by Sanchem, Inc., Chicago, Ill. immediately after fabrication to prevent corrosion, which shall be removed at the time of installation in the field. All finished surfaces shall also be protected by wooden lagging or other approved method, where applicable. All unfinished surfaces and finished non-mating surfaces shall be painted in accordance with the painting specification in this section of the proposal.

All shop drawings/submittals shall conform to Section 801.03 of the 2006 Edition of the “Louisiana Standard Specifications for Roads and Bridges.” Shop drawings shall be prepared directly by, or under the supervision of, the shop that will perform the work. Shop drawings shall not be prepared by an independent entity, and given to the shop for use in fabrication. The contractor shall obtain the dimensions of all manufactured products such as speed reducers, bearings, etc., from certified drawings provided by the manufacturer prior to the production of shop drawings. The weight of each fabricated or manufactured product shall be shown on the shop drawings.

The contractor may submit alternate material, manufacturers, equipment, or designs to the project manager for review and approval for the mechanical systems described in the plans and specifications. Alternates must be of equal or better quality as determined by the project manager. Approval of alternates does not guarantee they will interface properly with the existing mechanical designs or that the alternate will operate correctly once it is installed. The contractor is wholly responsible for the integration of the alternate into the existing mechanical designs. All re-designs to the mechanical systems to accommodate the use of the alternate shall be performed by the contractor at no additional cost to the LA DOTD. Shop drawings/submittals describing this re-design will be prepared by the contractor at no additional cost, and submitted to the project manager for review and approval.
All manufactured parts and equipment, in addition to all material for fabricated parts, shall be new. All manufactured parts and equipment in the specifications and plans can be considered to be followed by the phrase “or approved equal” unless otherwise stipulated in the plans or specifications.

FIELD WORK

All machinery pieces, including all fabricated and commercial items, which have been delivered to the field, but have not been installed, shall be stored in a weather tight structure/container and protected against rust and corrosion.

All machinery shall be installed by those competent and skilled in the type of work involved. Installers shall be provided with all the necessary measuring and leveling instruments as may be required.

Rust inhibitor coatings which protect finished surfaces of mechanical equipment shall be removed with gasoline or benzene at the time the equipment is being installed in its permanent configuration on the span. After installation/alignment is complete, lubricated surfaces shall be immediately cleaned and greased, and painted surfaces shall be given a touch-up coat of paint as per the paint specification.

All mechanical equipment aligned with shim packs shall be precisely adjusted to the tolerances provided by the shim packs.

After erection of machinery is complete, the contractor shall make a thorough inspection to ensure that all gears are clean and free of obstruction and that all bolts are properly tightened.

SPAN DRIVE SPEED REDUCERS (THREE (3) REQUIRED)

All gear reducer refurbishments/replacements shall be the product of a manufacturer of established reputation who has had gear reducers of comparable size, material, and type in successful service for at least ten (10) years. Unless given prior approval, all gearboxes shall be refurbished/replaced by one of the following gear reducer manufacturers:

3. FalkRenewPrager Co.; 472 Andrew Higgins Dr., New Orleans LA, 70161.

Gear reducers shall be designed in accordance with the current AGMA Standards and have a service factor of 2.0 for strength & 1.25 for durability. All reducers shall be installed outdoors and exposed to the weather under normal operation. The housing, accessories, seals, and protective finishes shall all be appropriate for such application.

HOUSING: Housing shall be constructed of steel with adequate inspection openings to permit easy inspection of all gears after installation. Inspection covers shall be attached with stainless steel hardware. Seals for inspection covers shall be appropriate for outdoor use.

Housing shall have a hygroscopic air breather appropriate for outdoor use with an adequate filter to prevent particulate matter from entering the housing.
Housing shall have an oil drain with a bronze or stainless steel drain valve. The valve shall have a bronze or stainless steel plug to prevent loss of the lubricant due to accident or vandalism.

Housing shall have a sight glass appropriate for outdoor use to measure the lubricant level.

The housing shall have a permanent, stainless steel, or aluminum name plate stating the name of the gearbox manufacturer, horsepower rating, service factors, input rpm, output rpm, gear ratio, thermal rating, and application name (such as Gearbox 1") to differentiate the gearboxes.

**GEARS:** Gears shall be cut and mounted to meet requirements for accuracy of the AGMA Standard 390.03. All gears shall be through hardened and shall have an AGMA Class 9 quality or better. The AGMA quality number shall be stated on the applicable shop drawings.

The teeth of all gears shall be cut from solid rims or blanks. The working surfaces of all teeth shall be true to the proper outline, accurately spaced on the pitch circle, free from planning or mill cutter ridges, cutter burrs shall be removed from all edges of teeth, and the top edges of teeth shall be rounded to 1/32" radius.

**INPUT SHAFTS:** The one (1) primary speed reducer shall have two (2) non-differential input shafts parallel with two (2) output shafts as shown on the plans. The two (2) secondary right-angle reducers shall have one (1) non-differential input shaft perpendicular with two (2) output shafts as shown on the plans.

The input shafts shall be designed for **two (2) times** the rated horsepower of the speed reducer.

The diameter and length of the input shafts shall be adequate such that they can be turned and keyseated for shaft couplings and brake wheels as shown on the plans.

The input shafts shall have balanced design double herringbone or double helical gears.

**OUTPUT SHAFTS:** The one (1) primary speed reducer shall have two (2) output shafts parallel to the input shafts as shown in the plans. The two (2) secondary speed reducers shall have two (2) output shafts perpendicular to the input shaft as shown in the plans.

The output shafts shall be capable of differential output, but shall also be capable of being locked together to act as one shaft by means of a manual clutch mechanism.

The diameter and length of the output shafts shall be adequate such that they can be turned and keyseated for shaft couplings and brake wheels.

**CLUTCH MECHANISM:** The clutch mechanism shall be engaged and disengaged by pushing and pulling an external rod as shown in the plans. The clutch mechanism shall be capable of locking and unlocking the output shafts whether or not the gear reducer is loaded, and whether or not the gear reducer is turning.

**SHAFT SEALS:** The input and output shafts shall have double nitrile rubber shaft seals with a provision to grease between the two (2) seals.
BEARINGS: The bearings shall be anti-friction type, selected to provide 100,000 hours B-10 life while transmitting the full rated motor horsepower.

LUBRICATION: The gear reducers shall be lubricated with a synthetic lubricant as recommended by the reducer manufacturer.

SOUND LEVEL: The sound level shall not exceed 90 db at a distance of 3 feet.

TESTING: All three (3) speed reducers shall be tested by the manufacturer in the presence of a representative of the LA DOTD before shipment to the shop or field for incorporation in the span drive machinery assembly. The gear reducer manufacturer shall provide a ten (10) day notice to the project manager of the date of the testing. All measurements taken during testing shall be recorded and submitted to the project manager for review and approval. No gear reducers shall be shipped to the field without being fully tested, calibrated, and inspected.

1. Speed Reducer Test Part 1, No Load Test (All Reducers):

Parts 1, 2, and 3 of the speed reducer tests shall be performed consecutively. The speed reducer manufacturer shall not start Part 1 unless he can complete all three (3) parts. Care shall be taken during planning of the testing so that the tests can proceed from Part 1 to Part 2 and from Part 2 to Part 3 with minimal delay.

Prior to the start of Part 2 of the testing, bluing shall be applied to all the gearsets so that contact percentage and pattern can be checked after the tests are complete.

Engage clutch (output shafts locked together), turn the input shaft at 900 rpm, no load on output shafts. Run speed reducer under no load condition for 30 minutes. During test, check and record sound levels all around gear reducer (90 db max. at 3 feet). Verify and record output rpm. At fifteen (15) minute intervals, measure and record external temperatures at each bearing. After 30 minutes, stop the gear reducer, and immediately open the access panel. Measure and record the temperature of all internal bearings.

Repeat test for opposite direction. If no issues are found, proceed immediately to Part 2.

2. Speed Reducer Test Part 2, No Load Test (All Reducers):

Disengage clutch (output shafts not locked together), turn the input shaft at 450 rpm, hold one output shaft fixed. Run speed reducer under no load condition for 30 minutes. During test, check and record sound levels all around gear reducer (90 db max. at 3 feet). Verify and record output rpm. At fifteen (15) minute intervals, measure and record external temperatures at each bearing. After 30 minutes, stop the gear reducer, and immediately open the access panel. Measure and record the temperature of all internal bearings.

Repeat test holding the other output shaft fixed. If no issues are found, proceed immediately to Part 3.
3. Speed Reducer Test Part 3, Load Test (All Reducers):

Engage clutch (output shafts locked together), turn the input shaft at 870 rpm, slowly apply load to one (1) output shaft until full load torque is reached. Run speed reducer for 30 minutes. During test, check and record sound levels all around gear reducer (90 db max. at 3 feet). Measure and record the output shaft rpm of both output shafts to verify that there is no clutch slippage. At fifteen (15) minute intervals, measure and record external temperatures at each bearing.

After 30 minutes, reverse the direction of the input shaft. Use same input speed and output torque. Run speed reducer for 30 more minutes. During test, check and record sound levels all around gear reducer (90 db max. at 3 feet). At fifteen (15) minute intervals, measure and record external temperatures at each bearing. After 30 minutes, stop the gear reducer and immediately open the access panel. Measure and record the temperature of all internal bearings.

Repeat the test using opposite output shaft. A visual inspection of all gear sets shall be performed. Contact patterns from the bluing shall be photographed and logged for proper identification.

SHIPMENT: After inspection and testing is complete, the speed reducers shall be transported to the shop. The entire gear reducer shall be cleaned, prepped, and painted in accordance with the paint specification.

**ROLLER BEARINGS**

All roller bearings shall be the product of a manufacturer of established reputation who has had bearings of comparable size, material, and type in successful service for at least ten (10) years.

**KEYS AND KEYWAYS**

Size, length, and fit of keys and keyways shall conform to 6.7.10 of the latest edition of the AASHTO “Movable Highway Bridge Design Specifications.” The ends of all keys shall be rounded to a half circle equal to the width of the key. All keys shall fill the entire keyway and be fabricated as per ASTM A 668, Class D material specification.

**SHIMS**

1. “A” SHIM PACKS: “A” Shim packs contain very fine shims and are capable of positioning equipment with a high degree of accuracy. They shall be used when appropriate for machinery that requires precise placement to function correctly (gear reducers, bearings, etc.). Commercially available, stainless steel, square, slotted, shim packs with an accuracy of .001” should be used.

The shop shall fabricate “A” shim packs for equipment of unusual shape, or for equipment that must bear load over a large area (pillow block bearings). Number and thickness of shims in each shim pack shall be as per the following table.
2. **“B” SHIM PACKS:** "B" shim packs have an accuracy of ± \(\frac{1}{16}\)". Machinery parts that do not require highly accurate positioning to operate properly shall be shimmed with “B" shim packs as indicated in the plans or specifications.

3. **SHIM PACK SUBMITTALS:** Cut sheets for commercial shim packs shall be submitted to the project manager for review and approval. Shop drawings of all fabricated shim packs shall be submitted to the project manager for review and approval. Shop drawings shall show dimensions, material, and number of shims supplied with each shim pack.

4. **WORKMANSHIP OF FABRICATED SHIM PACKS:** Shim packs fabricated by the shop shall neatly match the shape of the equipment to be shimmed. All slots/holes shall be pre-drilled and shall have the thickness of each shim marked on the shim with an indelible marker.

5. **PACKING/HANDLING OF SHIM PACKS SENT TO THE JOB SITE:**

   a. **PACKAGING:** Individual shim packs not installed in the shop shall be carefully packaged/bundled in a manner that will prevent damage of the loss of shims during shipping/handling.
Packaging shall be permanently labeled to reference the shim pack with the plans.

Shim packs shall then be packed together in a watertight, resealable container by the shop in a manner that prevents damage during shipping.

b. LABELING: The container shall be clearly and permanently labeled to identify the enclosed shim packs and shall reference the shim packs with the plans.

c. FIELD HANDLING: In the field, the container shall be stored out of the weather, and shall not be opened until the contractor is ready to install the shim packs. The contractor shall only remove from storage the shim packs that will be installed during a work shift.

PAINTING

1) All unfinished metal surfaces, finished non-mating metal surfaces, and galvanized surfaces shall be painted with two (2) prime coats and two (2) finish coats from one of the following paint systems.

<table>
<thead>
<tr>
<th>MOVABLE BRIDGE MACHINERY PAINT SYSTEMS</th>
<th>PAINT MANUFACTURER</th>
<th>EPOXY PRIMER</th>
<th>POLYURETHANE FINISH COAT</th>
</tr>
</thead>
<tbody>
<tr>
<td>DESIGNED FOR</td>
<td>CARBOLINE CO.</td>
<td>CARBOMASTIC 15</td>
<td>CARBOTHANE NO. 134 HS</td>
</tr>
<tr>
<td>ALTERNATE #1</td>
<td>AMERON COATINGS</td>
<td>AMERLOCK 400AL</td>
<td>AMERCOAT 450HS</td>
</tr>
<tr>
<td>ALTERNATE #2</td>
<td>PLAS-CHEM COATING CO.</td>
<td>CHEM-MASTIC 2214</td>
<td>CHEM-THANE 2810</td>
</tr>
</tbody>
</table>

2) Before application of the paint in the shop, surfaces which require painting must be cleaned of all chips, burrs, dirt, rust scale, sand, grease, and other extraneous materials by employing methods described in Section 811.06 of the Louisiana Standard Specifications, 2006 Ed. Galvanized surfaces shall be addressed according to paint manufacturer’s recommendations.

3) After cleaning, fabricated items that are not part of a shop assembly shall be given two (2) shop primer coats with time between coats as recommended by the paint manufacturer. After the paint has completely dried, the item shall be carefully packed and prepared for transportation to the field.

4) After cleaning, fabricated items that will be part of a shop assembly shall be given one (1) shop primer coat initially.

After the item has been incorporated into the shop assembly, and testing of the assembly is complete, the entire assembly, including all commercial items that are painted by their manufacturer (pillow blocks, reducers, couplings, etc.), shall be abraded and then given one primer coat.
The contractor shall take special care to avoid painting machinery bearing and sliding surfaces, shaft seals, and to mask and protect all name plates, legend plates, and escutcheons mounted on machinery.

All non-painted metal surfaces shall be given a new coat of grease at this time.

5) Immediately after machinery items have been installed in their final positions on the bridge, a touch-up coat of primer must be applied to all surfaces where the primer has been scratched or damaged and any other non-sliding metal surfaces which are exposed. These surfaces shall be cleaned of grease, oil, and loose materials by the use of solvents and compressed air. Touch-up primer must be the same as the original primer. This field touch-up painting shall be applied as recommended by the paint manufacturer. The contractor shall take special care to avoid the painting of machinery bearing and sliding surfaces, shaft seals, and to mask and protect all name plates, legend plates, and escutcheons mounted on machinery.

6) After field touch-up painting, all primed machinery surfaces shall be abraded and one finish coat applied. All commercial items that are painted by their manufacturer (pillow blocks, reducers, couplings, etc.) shall also be abraded and then given one finish coat. Name plates shall be clean and free of paint.

7) After completion of the operating tests and acceptance of the machinery, all oil, grease, dirt, and other foreign matter shall again be cleaned from the painted machinery surfaces then the final top coat of the approved paint system be applied. Final inspection of all painted machinery shall be conducted and touch-up primer and/or finish coat shall be applied where any scratches remain.

**LUBRICATION**

Prior to operating any machinery, the contractor shall prepare and furnish to the project manager for approval a lubrication diagram (for new equipment only) that shall include the following:

1. Location of each machinery part that requires lubrication. This shall be shown in relationship to the bridge structure.
2. Name of part.
4. Specific lubricant (manufacturer’s designation).
5. Frequency of lubrication.
6. Quantity (whether to change the lubricant of top off).

No machinery shall be operated without proper lubrication as specified in the approved diagram.

A copy of the approved diagram shall be framed, sealed, and mounted in the operator’s house as directed by the project engineer.

A copy of this approved diagram shall be made available to supplement the operation and maintenance manual.
Commercial products shall be delivered to the field lubricated in accordance with the approved diagram. A signed certification label indicating the type and date of lubrication shall be attached to and accompany all commercial items delivered to the field.

All routine lubrication shall be performed by the contractor until final acceptance of the bridge by the LA DOTD.

At final acceptance of the bridge, the contractor shall furnish 10 lbs. of each type of grease and 5 gal. of each type of oil required for the newly installed machinery. The contractor shall also provide two (2) permanently labeled lever guns with three (3) foot hoses for each type of lubricant.

When fittings are not mounted directly to the machinery, they shall be connected by ¼" stainless steel pipe extensions, standard weight and located in convenient and protected positions. Pipe extensions shall be kept as short as practical and shall be rigidly supported at the fittings and at intermediate points. Fittings on proprietary parts shall be replaced, if necessary, with fittings that have been selected as standard for the bridge.
<table>
<thead>
<tr>
<th>Control Section</th>
<th>Highway</th>
<th>Parish</th>
<th>TS No.</th>
<th>Sheet Of</th>
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<tr>
<td>BELLEVUE BRIDGE</td>
<td>S1 0</td>
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<td>GALLIANO BRIDGE</td>
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<tr>
<td>GALLIANO BRIDGE</td>
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<td>8.8 MILES AHEAD</td>
<td>ON TYPE III BARRICADE S3</td>
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<tr>
<td>GALLIANDITARPONI BRIDGE</td>
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<td>GOLDEN MEADOW BRIDGE</td>
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STATE OF LOUISIANA
DEPARTMENT OF TRANSPORTATION AND
DEVELOPMENT

CONSTRUCTION PROPOSAL
INFORMATION
FOR

STATE PROJECT NO. 407-01-0046

GALLIANO BRIDGE
GEARBOX REPAIRS

LOCATION: LAFOURCHE PARISH
LA 308 & BAYOU LAFOURCHE
**BID BOND**

A Bid Bond is required when the bidder’s total bid amount as calculated by the Department in accordance with Subsection 103.01 is greater than $50,000. *(See Section 102 of the Project Specifications.)*

________________________, as Principal (Bidder) and __________________________, as Surety,

are bound unto the State of Louisiana, Department of Transportation and Development, (hereinafter called the Department) in the sum of five percent (5%) of the bidder’s total bid amount as calculated by the Department for payment, of which the Principal and Surety bind themselves, their heirs, executors, administrators, successors and assigns, as solidary obligors.

Signed and sealed this ___________ day of ____________________________, 20_____.

The condition of this obligation is such that, whereas the Principal has submitted a bid to the Department on a contract for the construction of **STATE PROJECT NO. 407-01-0046, Galliano Bridge Gearbox Repairs**, if the bid is accepted and the Principal, within the specified time, enters into the contract in writing and gives bond with Surety acceptable to the Department for payment and performance of said contract, this obligation shall be void; otherwise to remain in effect.

<table>
<thead>
<tr>
<th>Principal (Bidder or First Partner to Joint Venture)</th>
<th>If a Joint Venture, Second Partner</th>
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<tr>
<td>By</td>
<td>By</td>
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<tr>
<td>Authorized Officer-Owner-Partner</td>
<td>Authorized Officer-Owner-Partner</td>
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Typed or Printed Name

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<th>Surety</th>
<th>If a Joint Venture, Second Partner</th>
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<td>By</td>
<td>By</td>
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Agent or Attorney-in-Fact

Typed or Printed Name

To receive a copy of the contract and subsequent correspondence / communication from LA DOTD, with respect to the bid bonds, the following information must be provided:

<table>
<thead>
<tr>
<th>Bonding Agency or Company Name</th>
<th>Address</th>
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<tr>
<th>Agent or Representative</th>
<th>Phone Number / Fax Number</th>
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07/07

Form CS-2A

H-1
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<th>APPROXIMATE QUANTITY</th>
<th>UNIT OF MEASURE</th>
<th>PAY ITEM UNIT PRICE (IN WORDS, INK OR TYPED)</th>
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<td>TEMPORARY SIGNS &amp; BARRICADES</td>
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<td>727-01</td>
<td>LUMP</td>
<td>LUMP SUM</td>
<td>MOBILIZATION</td>
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<td>LUMP</td>
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<td>REHABILITATION OF BRIDGE DRIVE SHAFT SYSTEM</td>
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<td>LUMP SUM</td>
<td>REMOVAL AND INSTALLATION OF GEAR BOXES</td>
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CONSTRUCTION PROPOSAL SIGNATURE AND EXECUTION FORM
THIS FORM, THE SCHEDULE OF ITEMS, AND THE PROPOSAL GUARANTY MUST BE COMPLETED AS INDICATED AND SUBMITTED
TO THE LOUISIANA DEPARTMENT OF TRANSPORTATION AND DEVELOPMENT (DOTD) TO CONSTITUTE A VALID BID

STATE PROJECT NO. 407-01-0046

FEDERAL AID PROJECT NO(S). N/A

NAME OF PROJECT Galliano Bridge Gearbox Repairs

I (WE) HEREBY CERTIFY THAT I (WE) HAVE CAREFULLY EXAMINED THE PROPOSAL, PLANS AND SPECIFICATIONS, INCLUDING
ANY AND ALL ADDENDA, AND THE SITE OF THE ABOVE PROJECT AND AM (ARE) FULLY COGNIZANT OF ALL PROPOSAL
DOCUMENTS, THE MASTER COPY OF WHICH IS ON FILE AT DOTD HEADQUARTERS IN BATON ROUGE, LA., AND ALL WORK,
MATERIALS AND LABOR REQUIRED THEREIN, AND AGREE TO PERFORM ALL WORK, AND SUPPLY ALL NECESSARY MATERIALS
AND LABOR REQUIRED FOR SUCCESSFUL AND TIMELY COMPLETION OF THE ABOVE PROJECT AND TO ACCEPT THE SUMMATION
OF THE PRODUCTS OF THE UNIT PRICES BID ON THE SCHEDULE OF ITEMS ATTACHED HERETO AND MADE A PART HEREOF
MULTIPLIED BY THE ACTUAL QUANTITY OF UNIT MEASURE PERFORMED FOR EACH ITEM, AS AUDITED BY DOTD, AS FULL
AND FINAL PAYMENT FOR ALL WORK, LABOR AND MATERIALS NECESSARY TO COMPLETE THE ABOVE PROJECT, SUBJECT TO
INCREASE ONLY FOR PLAN CHANGES (CHANGE ORDERS) APPROVED BY THE DOTD CHIEF ENGINEER OR HIS DESIGNEE. THIS BID
IS SUBMITTED IN ACCORDANCE WITH THE GENERAL BIDDING REQUIREMENTS IN THE CONSTRUCTION PROPOSAL AND ALL
SPECIAL PROVISIONS, PLANS, SUPPLEMENTAL SPECIFICATIONS, AND THE LOUISIANA STANDARD SPECIFICATIONS FOR ROADS
SCHEDULE OF ITEMS MULTIPLIED BY THE ESTIMATED QUANTITY OF UNIT MEASURE FOR EACH ITEM, ALONG WITH ANY
OTHER FACTORS SPECIFIED TO BE APPLICABLE SUCH AS CONSTRUCTION TIME AND/OR LANE RENTAL, SHALL BE THE BASIS FOR
THE COMPARISON OF BIDS. I (WE) UNDERSTAND THAT THE SCHEDULE OF ITEMS MUST CONTAIN UNIT PRICES WRITTEN OUT IN
WORDS AND THAT THE SCHEDULE OF ITEMS SUBMITTED AS PART OF THIS BID IS ON THE FORM SUPPLIED BY DOTD IN THE BID
PROPOSAL. MY (OUR) PROPOSAL GUARANTY IN THE AMOUNT SPECIFIED FOR THE PROJECT IS ATTACHED HERETO AS EVIDENCE
OF MY (OUR) GOOD FAITH TO BE FORFEITED IF THIS BID IS ACCEPTED BY DOTD AND I (WE) FAIL TO COMPLY WITH ANY
REQUIREMENT NECESSARY FOR AWARD AND EXECUTION OF THE CONTRACT, AS WELL AS, SIGN AND DELIVER THE CONTRACT
AND PAYMENT/PERFORMANCE/RETAINAGE BOND AS REQUIRED IN THE SPECIFICATIONS.

NONCOLLUSION DECLARATION (APPLICABLE TO FEDERAL-AID PROJECTS)
I (WE) DECLARE UNDER PENALTY OF PERJURY UNDER THE LAWS OF THE UNITED STATES AND THE STATE OF LOUISIANA THAT I (WE) HAVE
NOT DIRECTLY OR INDIRECTLY, ENTERED INTO ANY AGREEMENT, PARTICIPATED IN ANY COLLUSION, OR OTHERWISE TAKEN ANY ACTION IN
RESTRAINT OF FREE COMPETITIVE BIDDING IN CONNECTION WITH THE CONTRACT FOR THIS PROJECT NOR VIOLATED LA. R.S. 48:254.

BIDDER’S DBE GOAL STATEMENT (APPLICABLE TO DBE GOAL PROJECTS)
IF THIS PROJECT IS DESIGNATED BY SPECIAL PROVISION AS A DISADVANTAGED BUSINESS ENTERPRISE (DBE) GOAL PROJECT IN ACCORDANCE
WITH THE DBE PROVISIONS OF THIS CONTRACT, THE BIDDER ASSURES DOTD THAT HE/SHE WILL MEET OR EXCEED THE DBE CONTRACT GOAL,
OR IF THE BIDDER CANNOT MEET THE REQUIRED DBE GOAL, THE BIDDER ASSURES DOTD THAT HE/SHE HAS MADE AND CAN DOCUMENT GOOD
FAITH EFFORTS MADE TOWARDS MEETING THE GOAL REQUIREMENT IN ACCORDANCE WITH THE CONTRACT AND DBE PROGRAM MANUAL
INCORPORATED HEREIN BY REFERENCE.

CERTIFICATION OF EMPLOYMENT OF LOUISIANA RESIDENTS TRANSPORTATION INFRASTRUCTURE MODEL
FOR ECONOMIC DEVELOPMENT (TIME) PROJECTS (APPLICABLE TO TIME PROJECTS)
IF THIS PROJECT IS DESIGNATED BY SPECIAL PROVISION AS A TRANSPORTATION INFRASTRUCTURE MODEL FOR ECONOMIC DEVELOPMENT
(TIME) PROJECT AS DEFINED IN ACT NO. 16 OF THE 1989 FIRST EXTRAORDINARY SESSION OF THE LEGISLATURE WHICH ENACTED PART V OF
THE BIDDER CERTIFIES THAT AT LEAST 80 PERCENT OF THE EMPLOYEES EMPLOYED ON THIS TIME PROJECT WILL BE LOUISIANA RESIDENTS IN
ACCORDANCE WITH LOUISIANA R.S. 47:820.3.

NON PARTICIPATION IN PAYMENT ADJUSTMENT (ASPHALT CEMENT AND FUELS) STATEMENT
IF THIS PROJECT IS DESIGNATED By SPECIAL PROVISION AS BEING SUBJECT TO PAYMENT ADJUSTMENT FOR ASPHALT CEMENT AND/OR FUELS,
THE BIDDER HAS THE OPTION OF REQUESTING EXCLUSION FROM SAID PAYMENT ADJUSTMENT PROVISIONS THAT ARE ESTABLISHED BY
SPECIAL PROVISION ELSEWHERE HEREIN.

Failure to mark this box prior to Bid opening will constitute forfeiture of the Bidder's option to Request exclusion.

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STATE PROJECT NO. 407-01-0046

BIDDER SIGNATURE REQUIREMENTS (APPLICABLE TO ALL PROJECTS)

THIS BID FOR THE CAPTIONED PROJECT IS SUBMITTED BY:

Name of Principal (Individual, Firm, Corporation, or Joint Venture)

If Joint Venture, Name of First Partner

(Louisiana Contractor’s License Number of Bidder or First Partner to Joint Venture)

(Business Street Address)

(Business Mailing Address, if different)

(Area Code and Telephone Number of Business)

(Telephone Number and Name of Contact Person)

(Telecopier Number, if any)

If Joint Venture, Name of Second Partner

(Louisiana Contractor’s License Number of Second Partner to Joint Venture)

(Business Street Address)

(Business Mailing Address, if different)

(Area Code and Telephone Number of Business)

(Telephone Number and Name of Contact Person)

(Telecopier Number, if any)

ACTING ON BEHALF OF THE BIDDER, THIS IS TO ATTEST THAT THE UNDERSIGNED DULY AUTHORIZED REPRESENTATIVE OF THE ABOVE CAPTIONED FIRM, CORPORATION OR BUSINESS, BY SUBMISSION OF THIS BID, AGREES AND CERTIFIES THE TRUTH AND ACCURACY OF ALL PROVISIONS OF THIS PROPOSAL, INCLUSIVE OF THE REQUIREMENTS, STATEMENTS, DECLARATIONS AND CERTIFICATIONS ABOVE AND IN THE SCHEDULE OF ITEMS AND PROPOSAL GUARANTY. EXECUTION AND SIGNATURE OF THIS FORM AND SUBMISSION OF THE SCHEDULE OF ITEMS AND PROPOSAL GUARANTY SHALL CONSTITUTE AN IRREVOCABLE AND LEGALLY BINDING OFFER BY THE BIDDER.

(Signature)  (Signature)

(Printed Name)  (Printed Name)

(Title)  (Title)

(Date of Signature)  (Date of Signature)

CONTRACTOR’S TOTAL BASE BID $ __________________________

IT IS AGREED THAT THIS TOTAL, DETERMINED BY THE BIDDER, IS FOR PURPOSES OF OPENING AND READING BIDS ONLY, AND THAT THE LOW BID FOR THIS PROJECT WILL BE DETERMINED FROM THE EXTENSION AND TOTAL OF THE BID ITEMS BY DOTD.

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